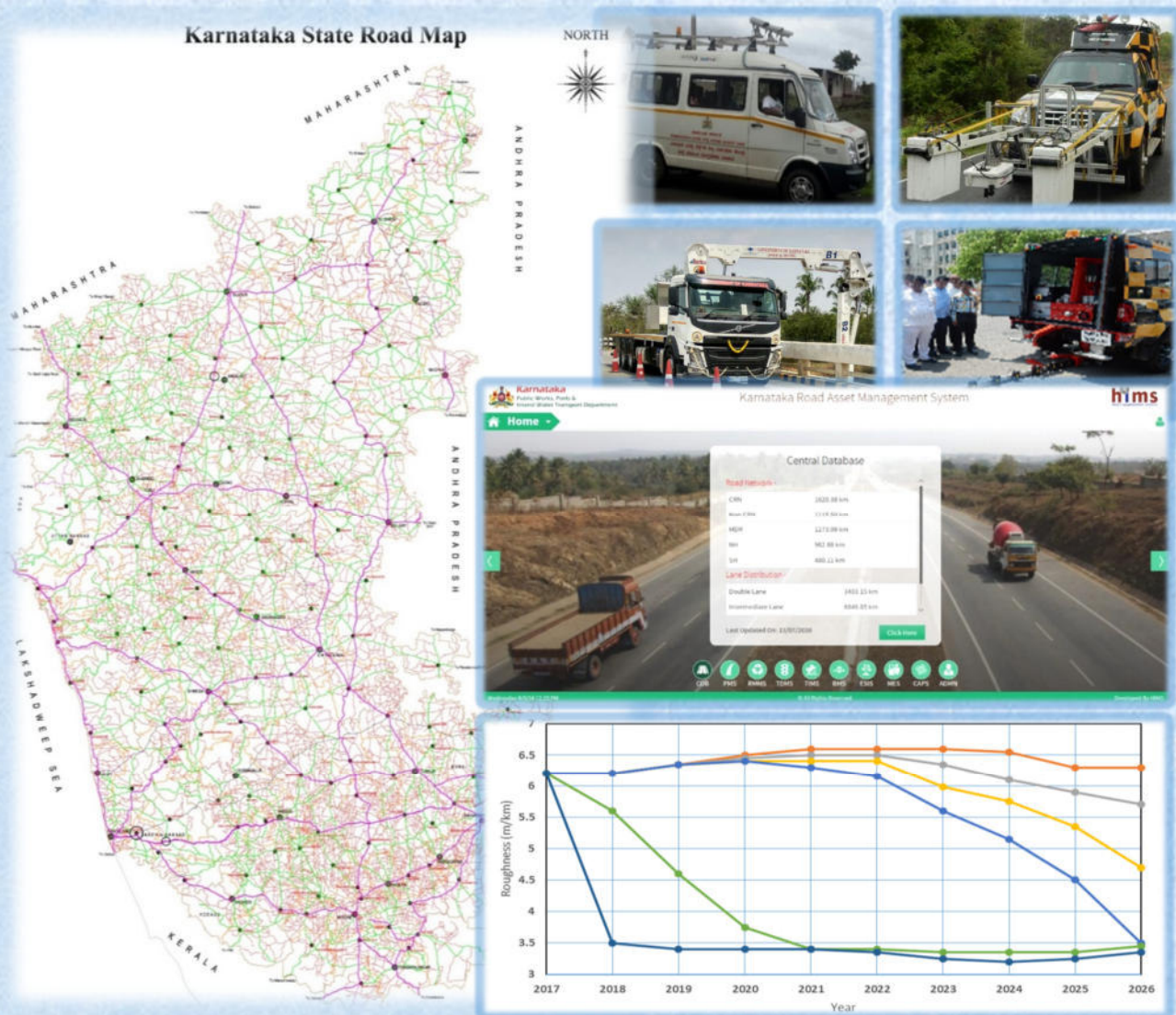




Government of Karnataka
Public Works Department

Road Data Collection, Data Processing and Quality Assurance Procedures



Planning and Road Asset Management Centre



Government of Karnataka
Public Works Department

ROAD DATA COLLECTION PROCEDURES



TABLE OF CONTENTS

1. INTRODUCTION.....	1
1.1 CORE ROAD NETWORK (CRN)	1
1.2 ROAD DATA COLLECTION	1
1.3 INVENTORY AND CONDITION DATA.....	2
1.4 PAVEMENT STRENGTH	2
1.5 PAVEMENT LAYERS INFORMATION.....	4
1.6 SCOPE OF THIS DOCUMENT.....	5
1.7 GENERAL	5
1.7.1 ROADS AND LINKS	5
1.7.2 DATA COLLECTION PLAN	5
2. INVENTORY AND CONDITION - ROMDAS SURVEY	7
2.1 INSTALLED / CONFIGURED ROMDAS COMPONENTS	7
2.2 MAIN COMPONENTS OF ROMDAS	9
2.2.1 POWER DISTRIBUTION BOX (PDB)	9
2.2.2 RACK MOUNTED HARDWARE INTERFACE	9
2.2.3 HIGH RESOLUTION DISTANCE MEASURING INSTRUMENT (HRDMI)	10
2.2.4 LASER PROFILOMETERS (ROUGHNESS AND TEXTURE DEPTH).....	10
2.2.5 LASER RUT MEASUREMENT SYSTEM (LRMS)	10
2.2.6 GPS ANTENNA AND RECEIVER.....	11
2.2.7 VIDEO CAMERAS - RoW AND PAVEMENT	11
2.2.8 CONFIGURABLE KEYBOARDS (INVENTORY AND VISUAL CONDITION).....	12
2.2.9 COMPUTER MONITOR, KEYBOARD	13
2.2.10 ROMDAS SOFTWARE	14
2.3 CALIBRATION AND VALIDATION OF EQUIPMENT'S	14
2.4 PRE-SURVEY ACTIVITIES.....	15
2.4.1 TRAINING SURVEY CREW.....	15
2.4.2 ITEMS CARRIED FOR SURVEY	15
2.5 STARTING ROMDAS SURVEY	16
2.6 INVENTORY AND VISUAL CONDITION DATA COLLECTION	19
2.6.1 LOCATION REFERENCING POINT (LRP) DATA.....	19
2.6.2 ROAD CENTRE LINE AND GPS DATA	20
2.6.3 VIDEO DATA- RIGHT OF WAY (RoW) AND PAVEMENT	20
2.6.4 ROAD INVENTORY	20



2.6.5	VISUAL CONDITION DATA	23
2.7	PAVEMENT ROUGHNESS AND TEXTURE DATA	25
2.8	PAVEMENT RUT DATA.....	26
2.9	DAILY CHECKS	27
2.10	SURVEY LOG	28
2.11	GENERAL	28
2.11.1	SURVEY DIRECTION.....	28
2.11.2	MULTI LANE ROADS	28
2.11.3	NAMING OF SURVEY FILES	28
3.	<u>FALLING WEIGHT DEFLECTOMETER (FWD).....</u>	<u>29</u>
3.1	GENERAL	29
3.2	MAJOR COMPONENTS OF FWD	30
3.2.1	LOAD CELL	30
3.2.2	LOAD PLATE.....	30
3.2.3	GEOPHONE SENSORS	31
3.2.4	SENSOR BAR	31
3.2.5	REMOVABLE WEIGHTS.....	31
3.2.6	FWD CONTROLLER	31
3.2.7	TEMPERATURE SENSOR – IN-GROUND TEMPERATURE	32
3.2.8	RUGGED LAPTOP	32
3.2.9	DATA ACQUISITION SOFTWARE	32
3.3	CALIBRATION AND VERIFICATION.....	33
3.3.1	RELATIVE CALIBRATION	33
3.3.2	DMI CALIBRATION	34
3.3.3	TEMPERATURE SENSOR VERIFICATION	34
3.4	SOFTWARE SETTINGS AND CONFIGURATION	36
3.4.1	SETTINGS	36
3.4.2	CONFIGURATION OF SCREEN LAYOUT.....	37
3.5	LOAD LEVELS.....	37
3.6	PREPARATION FOR FIELD SURVEY.....	37
3.6.1	IDENTIFICATION OF HOMOGENEOUS SECTIONS.....	37
3.6.2	TRAINING SURVEY CREW	38
3.6.3	ITEMS CARRIED FOR SURVEYS	38
3.7	SURVEY PROCEDURE.....	39
3.7.1	INTERVAL OF TESTS.....	39
3.7.2	DEFLECTIONS MEASUREMENT	39



3.7.3	TEMPERATURES MEASUREMENT	40
3.7.4	FILE NAMING.....	40
3.8	REGULAR CHECKS	41
3.8.1	CHECKS OF VEHICLE	41
3.8.2	FWD CHECKS.....	41
3.8.3	PRE-OPERATIONAL CHECKS FWD	42
3.8.4	POST OPERATIONAL CHECKS- FWD	42
3.9	DATA CHECKS DURING SURVEY	44
3.9.1	ROLL-OFF.....	44
3.9.2	NON-DECREASING DEFLECTIONS	44
3.9.3	OVERFLOW	45
3.9.4	LOAD VARIATION	45
3.9.5	DEFLECTION VARIATION.....	45
3.9.6	DEALING WITH DEFLECTION ERRORS.....	45
3.9.7	DEALING WITH LOAD ERRORS	47
3.10	CORRECTIONS OF COLLECTED DATA.....	47
3.11	TEST RESULTS – FWD	48
4.	<u>GROUND PENETRATING RADAR (GPR).....</u>	<u>50</u>
4.1	GENERAL	50
4.1.1	DIGITAL ANTENNA DRIVER (DAD) CONTROLLER.....	51
4.1.2	HORN ANTENNAS AND DUAL FREQUENCY ANTENNA	51
4.1.3	DISTANCE MEASUREMENT INSTRUMENT (DMI).....	52
4.1.4	RATCHET BELTS.....	52
4.1.5	DATA ACQUISITION SOFTWARE	52
4.1.6	RUGGED LAPTOP	53
4.2	SETUP OF ACQUISITION SOFTWARE.....	53
4.3	DAILY CHECKS	54
4.4	SURVEY PROCEDURE-GPR.....	55
4.4.1	CALIBRATION	55
4.4.2	SAMPLING INTERVAL.....	56
4.4.3	PROCEDURE OF DATA COLLECTION	57
4.5	TEST PIT	57
4.5.1	SAMPLING INTERVAL.....	57
4.5.2	DATA COLLECTION	57
4.6	ITEMS CARRIED FOR SURVEYS.....	57
4.7	ANALYSIS OF DATA	58



LIST OF TABLES

Table 2-1: Items carried for ROMDAS surveys.....	16
Table 2-2: Location Reference Points	19
Table 2-3: Road Inventory – Key code Configuration	21
Table 2-4: Pavement Defects Ranges – IRC.....	23
Table 2-5: Condition Key code Configuration- ROMDAS	24
Table 3-1: Components- FWD Machine	29
Table 3-2: Items carried for FWD Survey	38
Table 3-3: Results Table- FWD.....	49
Table 4-1: Items carried for GPR and Test Pit Surveys	57

LIST OF FIGURES

Figure 1-1: ROMDAS Equipment	2
Figure 1-2: In-vehicle mounted- FWD	3
Figure 1-3: GPR Arrangement	4
Figure 2-1: Schematic Layout -ROMDAS.....	8
Figure 2-2: Power Distribution Box (PDB)	9
Figure 2-3: Rack Mounted Hardware Interface.....	9
Figure 2-4: HR- DMI.....	10
Figure 2-5: Laser Profilometers.....	10
Figure 2-6: LRMS Arrangement	11
Figure 2-7: GPS Antenna.....	11
Figure 2-8: Trimble GPS receiver.....	11
Figure 2-9: Right of Way (RoW) Cameras.....	12
Figure 2-10: Pavement Camera	12
Figure 2-11: Configurable Keyboard	13
Figure 2-12: Computer Monitor, Keyboard	13
Figure 2-13: ROMDAS Data Acquisition Software - Screen	14
Figure 2-14: Survey Start up /Setup Screen.....	18
Figure 2-15: End of Survey Screen	18



Figure 2-16: Labelling of Inventory Keyboard.....	21
Figure 2-17: Labelling of Visual Condition Keyboard.....	24
Figure 3-1: FWD- in-vehicle mounted.....	30
Figure 3-2: Load Cell.....	30
Figure 3-3: Load Plate.....	30
Figure 3-4: Geophone Sensor.....	31
Figure 3-5: Sensor Bar with Geophones	31
Figure 3-6: Removable Weights.....	31
Figure 3-7: FWD Controller	32
Figure 3-8: Sensor for In-ground Temperature.....	32
Figure 3-9: Rugged Laptop	32
Figure 3-10: Data Acquisition Software- FWD	33
Figure 3-11: FWD Settings.....	36
Figure 4-1: GPR setup on Vehicle.....	50
Figure 4-2: DAD Controller.....	51
Figure 4-3: Horn Antenna.....	51
Figure 4-4: Dual Frequency Antenna	51
Figure 4-5: DMI	52
Figure 4-6: Ratchet Belts	52
Figure 4-7: Data Acquisition Software-Start up Screen	53
Figure 4-8: Rugged Laptop	53
Figure 4-9: Spy lights	54
Figure 4-10: Calibration is underway.....	55
Figure 4-11: Acquisition Selection window	56
Figure 4-12: Acquired Data	56



ABBREVIATIONS

CRN:	Core Road Network
DAD:	Digital Antenna Driver Controller
DGPS	Differential Geographic Positioning System
DMI:	Distance Measuring Instrument
FWD:	Falling Weight Deflectometer
GAGAN:	GPS Aided Geo Augmented Navigation
GPR:	Ground Penetrating Radar
GPS:	Geographical Positioning System
HRDMI:	High Resolution Distance Measuring Instrument
IRI:	International Roughness Index
LRMS:	Laser Rut Measurement System
LRP:	Location Referencing Point
MDR:	Major District Road
NH:	National Highways
PDB:	Power Distribution Box
PDOP:	Position Dilution Of Precision
PRAMC:	Planning and Road Asset Management Centre
PWD:	Public Works Department
RIS:	Road Information System
ROMDAS:	ROad Measurement Data Acquisition System
RoW:	Right of Way
SH:	State Highway



FOREWORD

The Government of Karnataka has received support from the Asian Development Bank (ADB) for the establishment of the Planning and Road Asset Management Centre (PRAMC) in the Public Works, Ports and Inland Water Transport Department (PWP&IWTD – PWD in short). The PRAMC functions include, Policy Development, Planning and Programming, Road Safety, Information Technology support and Training support across the whole of PWD by introducing state-of-the-art technologies. In order to perform its mandated functions PRAMC has identified and procured the advanced technological road data collection equipments as mentioned below

- ROad Measurement Data Acquisition System (ROMDAS)
- Falling Weight Deflectometer (FWD)
- Ground Penetrating Radar (GPR).

ROad Measurement Data Acquisition System (ROMDAS): ROMDAS is helpful in collecting road inventory, road condition, roughness and texture depth, rut, road alignments, along with video footages.

Falling weight Deflectometer (FWD): This equipment is used by civil engineers to evaluate the physical properties of pavement including elasticity moduli, the bearing capacity, and pavement deflection under standard axle load. FWD data is primarily used to assess the strengthening requirements of the existing pavement.

Ground Penetrating Radar (GPR): This is a geophysical equipment that uses radar pulses to image the subsurface. This non-destructive method uses electromagnetic radiation in the microwave band of the radio spectrum, and detects the reflected signals from subsurface structures. Under PRAMC, GPR is used to assess the profiles of the underlying pavement layers and their thicknesses.

These equipments are mandated to be part of the regular data collection activities of PWD by following pre-established data collection procedures across the state. For this purpose, this document which gives insight of the utilities of the aforesaid equipment and provide guidelines about pre-survey, survey and post-survey activities, is being produced. However, it is to be noted that this document is not a replacement of the equipment's *User Manuals*, which are to be thoroughly referred for understanding the detailed concepts of equipments.



1. Introduction

1.1 Core Road Network (CRN)

Planning and Road Asset Management Centre (PRAMC) is responsible for Planning, Programming and monitoring the road maintenance works on the PWD road network. Within the overall road network of the state, the Government of Karnataka (GoK) has identified an initial Core Road Network (CRN) of approximately 25,000 km. This CRN comprises the mix of road links of National Highways (NH), State Highways (SH), Major District Roads (MDRs), which are considered most important for the economic and social development of the State. The composition of the road network CRN is as below

Classification	Length(Km)
National Highways	4,491
State Highways	18,470
Major District Roads	1,265
Total length	24,226

Source: ADB-CDTA-1

As a part of its mandated functions, PRAMC has been collecting several kinds of road data which is very much essential for determining the maintenance requirements of the CRN through state-of-the-art technologies.

1.2 Road Data Collection

Road data collection is critical to success of any road asset management, but too much inaccurate, unfriendly data is worse than having none at all. Hence, it is required to identify and collect the data which is actually used while preparing the annual maintenance / improvement programs.

During the inception stages PRAMC has identified the Road Data collection requirements along with the method of data collection. This data required to be collected, stored, managed, and analysed at regular intervals using the established method of data collection procedures. The overall road data collection requirements under PRAMC are broadly classified as

- Road Inventory and Condition Data;
- Pavement Strength;
- Pavement layers information;
- Video surveys.



Some of the above mentioned data is being collected by PWD as part of the regular data collection, however, the technologies and methods being followed are of outdated in nature and mostly manual in process. Hence, PRAMC has opted to procure advanced instruments for collecting the road data more accurately and with less human intervention. The following are the equipment procured by PRAMC for collecting disparate data

- Road Inventory ,Condition data and Video Surveys (**ROad Measurement Data Acquisition System –ROMDAS**)
- Pavement Strength (**Falling Weight Deflectometer -FWD**)
- Pavement Layers Details (**Ground Penetrating Radar -GPR**).

1.3 Inventory and Condition Data

The Road Inventory and Pavement Condition Survey of CRN roads are carried out using **ROad Measurement Data Acquisition System (ROMDAS)** Figure 1-1). ROMDAS comprises of the following components



Figure 1-1: ROMDAS Equipment

1. Distance Measuring Instrument (DMI)
2. Laser Profilers
3. LRMS (Laser Rut Measurement System)
4. GPS receiver
5. Video Cameras (Right of Way and Pavement)

Detailed elaborations of these equipments are given in the ensuing sections of this report.

1.4 Pavement Strength

Pavement strength is one of the most important parameters for determining the residual life of pavement and subsequently to estimate the strengthening requirements of the pavement (overlay



requirements) to make the pavement functional till the design life of the pavement. Pavement strength is assessed by measuring pavement deflection under a pre-determined axle load. Earlier days the pavement deflections are measured by using Benkelman Beam Deflection (BBD) technique, however, this method is tedious and more susceptible to human errors. In order to overcome these issues, internationally recognised advanced equipments such as Falling Weight Deflectometer (FWD) came into existence. The FWD is a non-destructive equipment and measures the vertical deflection response of a surface to an impulse load using the automated methods. The FWD equipment comprises of an arrangement to simulate the dynamic load from truck on the pavement. The information collected using FWD can be used to determine (i) Bearing Capacity (ii) Layer Thicknesses (iii) Elasticity Moduli and (iv) Residual life (expected surface life).

Hence, PRAMC has procured an in-vehicle mounted FWD which is fixed on Tata Xenon 4x4 vehicle. The major components of FWD equipment are shown in Figure 1-2.

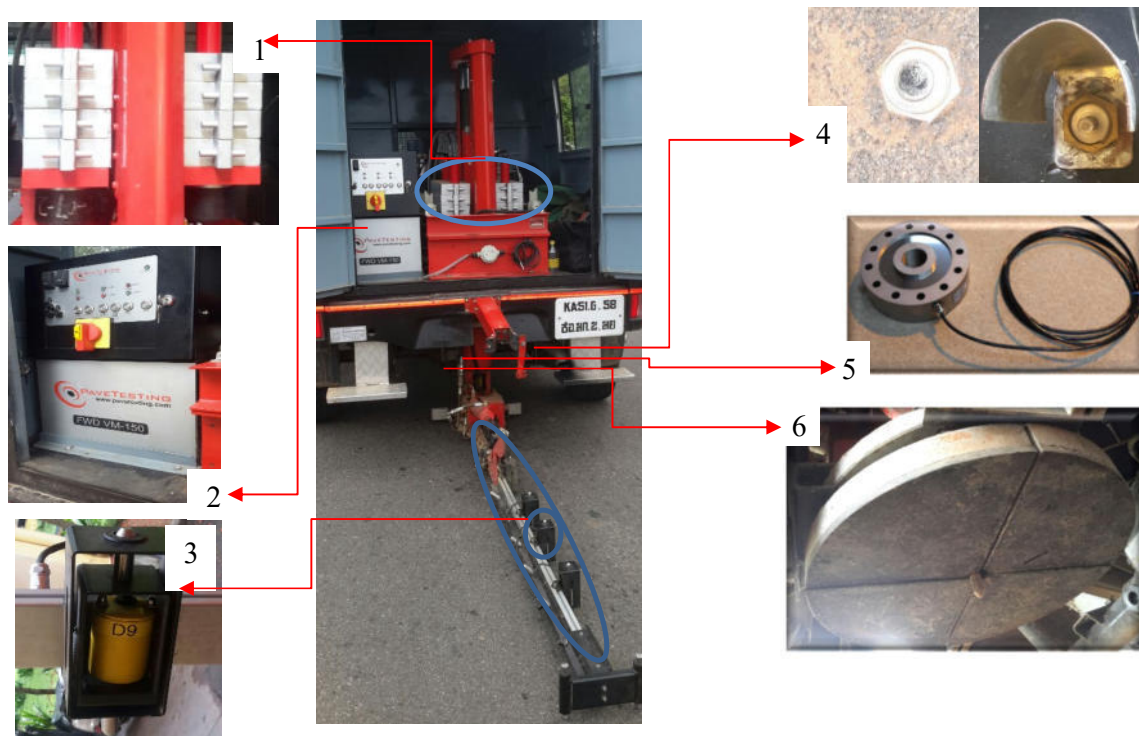


Figure 1-2: In-vehicle mounted-FWD

1. Weight Pack
2. Controller
3. Geophone Sensors (9Nos)
4. Temperature Sensors
5. Load Sensor / Cell



6. Load Plate

1.5 Pavement Layers Information

The existing pavement layers information is very critical in assessing the load bearing capacity of the pavement and also to identify the defects in the construction. The pavement layers information is used to be obtained by a destructive test of digging test / trail pits at select locations those are identified as per the homogeneity of the pavement condition.

In the recent past many technologies such as geophysical methods that uses radar pulses to image the subsurface came into existence. Initially these methods are not widely used in roads sector, however, after realising the benefits of these methods, their uses are extended to the road sector also and these methods are internationally accepted and proven successful in road sector. PRAMC has procured Ground Penetrating Radar (GPR) for identification of the pavement layers and their thicknesses. GPR is a non-destructive method and uses electromagnetic radiation in the microwave band of the radio spectrum, and detects the reflected signals from subsurface structures, which can be used to identify the pavement layer profiles, thicknesses. This GPR equipment is mounted on the front side of vehicle in which FWD is in-vehicle mounted (Figure 1-3).



Figure 1-3: GPR Arrangement

The major components of the GPR equipment are

1. Horn Antennas (1 Ghz and 2 Ghz)
2. Dual Frequency Antenna (900/400 Mhz)
3. Digital Antenna Driver(DAD) Controller
4. Distance Measurement Instrument (DMI)
5. Global Positioning System(GPS)





1.6 Scope of this Document

The objective of this “Road Data Collection Procedures” document is to provide instructions to the data collection teams about various activities related to the field data collection. This document includes the following;

- General Guidelines of Data Collection;
- Road inventory and condition data collection- ROMDAS;
- Pavement strength data collection- FWD;
- Pavement composition data collection- GPR;

1.7 General

1.7.1 Roads and Links

Road identification information is stored by PWD in the existing Road Information System (RIS) database. In RIS, the entire road network of the Karnataka has been broken down at the Taluk level and the road section between the taluk boundaries is named as Road. These roads of each taluk are further broken down into smaller road sections called links.

In RIS each road and link are given a unique six character identifier on the basis of Taluk. For example roads in Kolar Taluk are designated KOLXXX where KOL is the Taluk identification and XXX is a unique 3 digit road number. These Road IDs and Links IDs forms as a base to identify the roads, links for data collection activities.

1.7.2 Data Collection Plan

As mentioned else where the CRN consists of National Highways (NH), State Highways (SH) and Major District Road (MDR). In order to prepare data collection plans, the existing GIS maps, RIS data, google earth imageries shall be used to the possible extent. The data available to date includes a list of all roads and links, the road -link length, Taluk and District maps. The plans shall be prepared in such a way that maximum road length can be surveyed with minimum dry run. During the initial stages a daily output of 60-70 lane km/day on the paved roads network shall be kept as target, and later it can be increased when the survey teams get fully acquainted with the prevailing conditions of the road network. The road data aimed to collect on various road categories is as follows



Road Data Collection Procedures

Planning and Road Asset Management Centre (PRAMC)

Sr. No	Road Category	Types of Data
1	National Highways	Location Reference Points(LRP's), Road alignment
2	State Highways	Road Inventory and alignment, Road Condition, and LRP's
		Pavement Strength
		Pavement Layers details
3	Major District Roads	Road Inventory and alignment, Road Condition, and LRP's
		Pavement Strength
		Pavement Layers details



2. Inventory and Condition - ROMDAS Survey

2.1 Installed / Configured ROMDAS Components

ROMDAS system is configured with various equipments for collecting inventory and condition data. The equipments/ accessories installed on the ROMDAS survey vehicle are as mentioned below

- ROMDAS Hardware Interface;
- Power Distribution box;
- Odometer sensor to measure the distance;
- Laser Profilometers to measure roughness;
- Laser Rut Measurement System (LRMS) to measure rut depth;
- GPS receiver for capturing spatial data;
- Video Cameras for Right of Way (RoW) and Pavement;
- 58 key configurable ROMDAS keyboard for inventory and visual condition
- Computer monitor;
- Other cable connections.

The guidelines related to the equipments installation are provided in the respective sections / chapters of *ROMDAS User's Guide* which was supplied along with ROMDAS systems. The schematic representation of ROMDAS equipment is as shown in Figure 2-1.



Road Data Collection Procedures

Planning and Road Asset Management Centre (PRAMC)

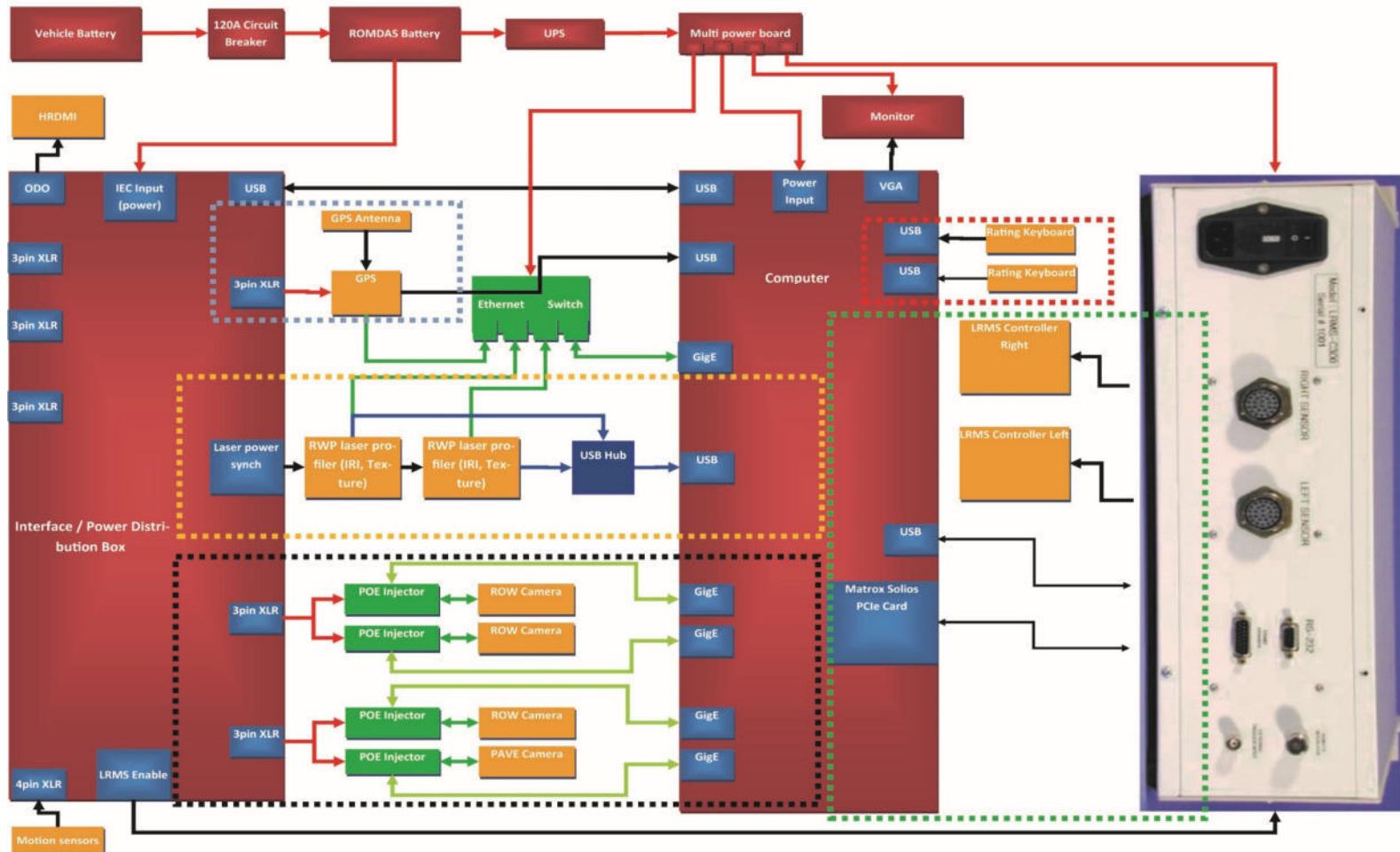


Figure 2-1: Schematic Layout -ROMDAS



2.2 Main Components of ROMDAS

2.2.1 Power Distribution Box (PDB)

There are multiple ROMDAS instruments which need power supply for the functioning. For this purpose Power Distribution Box (PDB) plays an important role in supplying power to different hardware components of ROMDAS and to the computer. ROMDAS is configured with a PDB as shown in Figure 2-2.



Figure 2-2: Power Distribution Box (PDB)

2.2.2 Rack Mounted Hardware Interface

The rack mount hardware interface is used for storing data and for operation of ROMDAS software and provide communication ports for the hardware components of the ROMDAS (Figure 2-3). This rack mounted hardware interface is mounted on the middle of the vehicle floor, such that it is in the easy access of surveyors for operation and cross checking.



Figure 2-3: Rack Mounted Hardware Interface



2.2.3 High Resolution Distance Measuring Instrument (HRDMI)

The High Resolution DMI sensor (HR DMI) is equipped with the ROMDAS vehicles for measuring high resolution distance which is required for LRMS, Laser Profilometers, and GPS.



Figure 2-4: HR- DMI

2.2.4 Laser Profilometers (Roughness and Texture Depth)

The Laser Profilometers uses laser and accelerometer combination to measure the longitudinal elevation of the profile and road texture characteristics. The output of these profilers gives raw longitudinal profile and International Roughness Index (IRI) in m/km. Under PRAMC project, two profilometers are installed on ROMDAS vehicle, one profiler for each wheel path, for measuring the roughness and texture of the pavement (Figure 2-5). The lasers used in the profilometer are of Class 3B and it is controlled by Laser Interface key. The profilometers are connected to the Rack Mounted Computer / hardware interface via an Ethernet connection and powered through the Laser DMI Interface which is controlled by the keyed switch.



Figure 2-5: Laser Profilometers

2.2.5 Laser Rut Measurement System (LRMS)

The ROMDAS Laser Rut Measurement System (LRMS) is a transverse road profiling system which is fixed at the rear of the vehicle as shown in Figure 2-6. LRMS consists of two lasers covering carriageway width of 4m.



Figure 2-6: LRMS Arrangement

The two LRMS lasers are responsible for the measurement of the left and right side transverse profiles. Each unit is composed of a high-power laser line projector and a special camera to measure deformations of the laser line profile. The lasers used in the LRMS are of Class 3B and it is controlled by key on Laser controller.

2.2.6 GPS Antenna and Receiver

The GPS antenna (Figure 2-7) is mounted on vehicle roof which catches the signals from satellites and sends to the receiver to record the spatial coordinates. GPS receiver (Figure 2-8) is used to record the spatial coordinates of various road attributes. Trimble SPS 461 receiver is used in ROMDAS.



Figure 2-7: GPS Antenna



Figure 2-8: Trimble GPS receiver

The antenna distance, laterally from the side of the vehicle and height above the ground level are measured and stored in the GPS settings and Trimble settings. These distances will be used to correct the data recorded so that the recorded coordinates represent the actual (true) road centre line at ground level.

2.2.7 Video Cameras - RoW and Pavement

4 Nos of Point Grey Research (PGR) Zebra cameras (3 cameras for capturing Right-of-Way (RoW) details and 1 camera for pavement view capturing) are installed on the survey vehicle.



The cameras are mounted on the survey vehicle roof and positioned in such a way that they capture a good general view of the road ahead, the roadside furniture in sufficient detail and the pavement surface view. These video footages can be used to identify inventory of various road parameters during post processing of data. RoW video features are captured with 2 MP (1600 x 1200) resolution at 25 frames per second and Pavement surface video is captured with 5 MP (2448 x 2048) at 18 frames per second (mono) resolution. Video cameras are enabled with image stabiliser to negate the effect of the vehicle vibrations.



Figure 2-9: Right of Way (RoW) Cameras



Figure 2-10: Pavement Camera

The PGR camera system connects to the survey computer through Ethernet cable via Ethernet Hub. The camera is powered through the power injector and the video data is compressed using Pegasus Pic Video M-jpeg Codec compression software.

2.2.8 Configurable Keyboards (Inventory and Visual Condition)

ROMDAS is equipped with two configurable keyboards having 58 programmable keys. One keyboard is used to record the inventory data while the other keyboard to record visual condition data. These keyboards are connected directly to the survey computer through a USB port. In ROMDAS system all the road data are grouped into events, which are named as (1) Point event, (2) Continuous event and (3) switch event

- **Point Events-** which something exists at a single location – e.g. km Post, traffic sign
- **Continuous Events** - spread over a section and are characterised by a start and end chainage- e.g., pavement condition, medians
- **Switch Events-** Special type of continuous events which are called as series of continuous events. These events are dependent on the other events within the same group. Generally surveyor would require to press two keys when changing the continuous events- one key is to end the previous condition and the other key is to apply the new condition. Switch events remove the need to press two keys. When the second key is



pressed the first event is cancelled automatically. Some of the switch events are, width standard, pavement condition etc.

As per the PRAMC data collection requirements the keys of each of these keyboards are programmed /configured and labelled (Figure 2-11). These configurations can be changed at any point of time in the future, as per then requirement. More details about the types of data events and the detailed procedure of configuring the keyboard keys is given in *ROMDAS User's guide- Appendix G*.



Figure 2-11: Configurable Keyboard

Visual Condition Data: The surveyor assesses the condition of the pavement in the survey direction over a 500 m section length and determines the condition value for each of the relevant parameters. The software prompts the surveyor with a series of beeps prior to the end of the 500m interval that the assessed condition data to be keyed in. On dual carriageways both increasing and decreasing lanes will be surveyed, therefore the data recorded refers to the inventory and condition data in the survey (travel) direction. For single carriageways the data refers to entire roadway.

2.2.9 Computer Monitor, Keyboard

A computer monitor is mounted at the middle of the vehicle. It is configured as a dedicated data collection computer. Power saving and screen saver options are turned off so that these do not hamper the progress of the data collection. Operations of various equipments and monitoring of the data collection will be performed through this monitor. This computer should be kept as “clean” as possible by avoiding installing more software which are not required for running ROMDAS operations. The power supply for the computer is drawn from the vehicle battery through ROMDAS power distribution box. Computer keyboard will be used for recording the text comments and LRPs during the data collection.



Figure 2-12: Computer Monitor, Keyboard



The Rack mounted computer is configured with the following specifications

- i7 Processor;
- 8GB RAM Memory;
- 320 GB Internal SSD Drive;
- 3X2TB removable HDD for data storage and transfer;
- Windows-7 (professional) operating system;
- MS Office;
- Drivers and software required for operating ROMDAS system.

2.2.10 ROMDAS Software

ROMDAS system is equipped with the ROMDAS data acquisition software. This software is configured to meet the data collection requirements of PRAMC. These settings should not be modified by the surveyors in the field unless such need arises. These configuration settings are saved in storage, and shall be replicated during reinstallation of software, if such situation arises.

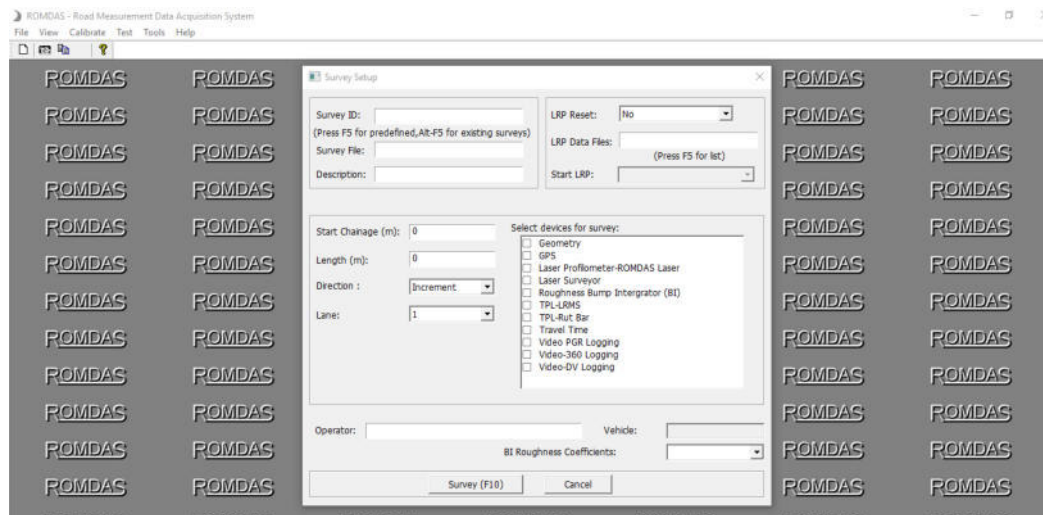


Figure 2-13: ROMDAS Data Acquisition Software - Screen

ROMDAS software is supplied with two modes of operations

ROMDAS Registered Version –is for data collection in the field; enabled for subsystems (equipments) purchased and installed in the computer of the survey vehicle;

ROMDAS Office Version – for office uses, this software is used for data processing, survey file setup etc.

2.3 Calibration and Validation of Equipment's

Prior to conducting any surveys it is necessary to calibrate the instruments / sub-systems. The following calibrations must be performed.



Odometer Calibration: This calibration ensures that the distance and speeds measured by the survey vehicle are correct. This calibration is to be performed at a frequency of **every 5000km** of survey. The procedure for the Odometer calibration is given in the *Calibrating the Instruments* section of the *ROMDAS User's Guide*.

LRMS Calibration and Verification: The LRMS installed on the survey vehicle are pre-calibrated before supply, which will not change under normal circumstances. However, this calibration can be verified or checked by using the *LRMS Calibration Object*. The procedure of verification or checking is provided in the *ROMDAS User's Guide* under Section 12.7: *LRMS Calibration Verification and Cleaning* section.

Laser Profilometer

- **Calibration:** The calibration of Laser Profilometer involves calibration of *Laser Odometer calibration* this defines the laser sampling distances, and the minimum speed laser safety cut-out. The detailed procedure of *Laser Odometer calibration* is given in the *ROMDAS User's guide*. The equipment used under PRAMC is pre-calibrated by the supplier. If this calibration test does not meet the requirements or the Profilometer or it is past its calibration expiry date, the Profilometer should be returned for calibration
- **Validation:** The accuracy of the Profilometer must be monitored with the Bounce checks. This measurement checks that all aspects of the Profilometer are working optimally and should be carried out daily before starting survey. The detailed procedure of Laser validation is given in the *ROMDAS User's guide*.

2.4 Pre-survey Activities

2.4.1 Training survey crew

Prior to proceeding to the field surveys, the survey crew shall be given training in all activities related to equipment operations including inventory data entry, condition assessment and condition data entry. Upon completion of training a sample survey shall be performed by the team members under the supervision of the trainers. This exercise is very much required to assess the understanding levels of the survey team and to provide clarifications on the issues, which they may face during the survey.

2.4.2 Items carried for survey

The below mentioned items are necessary for conducting the field surveys shall be carried to the field

**Table 2-1: Items carried for ROMDAS surveys**

Sr. No	Description	Remarks
1.	CRN Road Map	Printed copy
2.	CRN road map electronic copy	On survey computer
3.	ROMDAS Manuals	Printed copies
4.	Survey vehicle with Survey Equipment	For Conducting survey
5.	Back up External Drive (2TB)	Storing and Back up data
6.	Measuring Tape	30m length
7.	Pressure gauge	To check tyre pressure
8.	Traffic cones	To warn ongoing traffic during survey vehicle in stationary position
9.	Mobile phone with charger	For communication
10.	Tool Kit	To address small repairs
11.	Internet Dongle(3G Capability)	For communication
12.	Survey Log	For recording the issues in the field
13.	Note Book / Note Pad	Recording the issues, status of work
14.	Highlighters	For marking on the maps

2.5 Starting ROMDAS survey

Survey vehicle should be positioned at least 100 m before the survey section start and open the ROMDAS software application which is installed on the computer of the survey vehicle. This extra warmup length would enable the vehicle to get minimum speed by the time it reaches the actual survey section start and also to condition the input filtering system. Upon opening ROMDAS software application a Start-up window will open with five different menu options as follows:

- File – Survey setup parameters and data processing;
- View – View processed data;
- Calibrate – calibrate the survey components;
- Test – test the instruments connected;
- Tools – Define Key code events and survey IDs, LRP utilities;

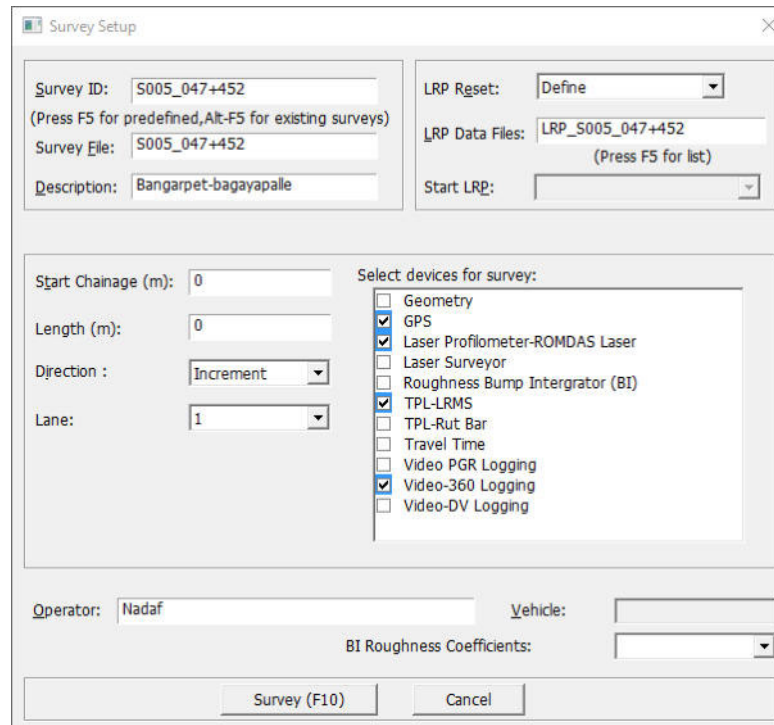


Detailed descriptions about the above mentioned functions are given in *ROMDAS User's guide*. Test all equipment for proper operation as per the guidelines of *ROMDAS user's guide* and start the new survey by defining survey data as mentioned below:

Select **File/New Survey** and the survey setup screen appears (Figure 2-14). Enter the appropriate information in the relevant fields as mentioned below

- Survey ID – A unique code identifying the survey, in the PRAMC survey use the link name;
- Survey File – This defaults to the survey Id and should not be changed unless a repeat survey or part survey is required;
- Description– a brief description of the survey, for example from intersection with SH6 to road end at junction with SH2;
- Set *LRP Reset* to *Define*;
- Select devices for survey – these would be normally enabled automatically from the settings in **Default to use in Survey** fields for each instrument
- Enter the start chainage – this will be as per the details of the survey section;
- Enter equipment operator and vehicle details.

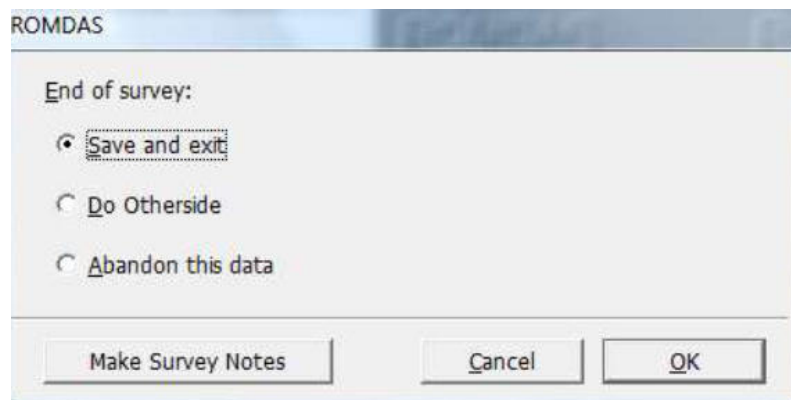
Once all the survey setup data is input, select "Survey (F10)" or press "F10" to get the survey screen and to commence the survey. At the completion of the survey, press the F10 key, which will open a window as shown in Figure 2-15.



The Survey Setup dialog box contains the following fields and options:

- Survey ID:** S005_047+452
- Survey File:** S005_047+452
- Description:** Bangarpet-bagayapalle
- LRP Rset:** Define
- LRP Data Files:** LRP_S005_047+452
- Start LRP:** (Empty dropdown)
- Start Chainage (m):** 0
- Length (m):** 0
- Direction:** Increment
- Lane:** 1
- Select devices for survey:**
 - ☐ Geometry
 - ☒ GPS
 - ☒ Laser Profilometer-ROMDAS Laser
 - ☐ Laser Surveyor
 - ☐ Roughness Bump Integrator (BI)
 - ☒ TPL-LRMS
 - ☐ TPL-Rut Bar
 - ☐ Travel Time
 - ☐ Video PGR Logging
 - ☒ Video-360 Logging
 - ☐ Video-DV Logging
- Operator:** Nadaf
- Vehicle:** (Empty text box)
- BI Roughness Coefficients:** (Empty dropdown)
- Buttons:** Survey (F10), Cancel

Figure 2-14: Survey Start up /Setup Screen



The ROMDAS End of Survey dialog box contains the following options and buttons:

- End of survey:**
 - ☒ Save and exit
 - ☐ Do Otherside
 - ☐ Abandon this data
- Buttons:** Make Survey Notes, Cancel, OK

Figure 2-15: End of Survey Screen

Sometimes the operator has something to note about the survey such as the details of the end location of the survey etc. The **Make Survey Notes** button allows user to enter comments on the survey when the survey ends. This data is stored in the Memo field of the *Survey_Header* table in the output file. It is advised to end the day survey at such locations those can be easily identified while resuming the survey the next day. These locations could be junctions or villages or any other permanent structure.



2.6 Inventory and Visual Condition Data Collection

As part of Inventory and visual condition data collection the following data required to be collected.

- Location reference point (LRP) data;
- Road inventory data;
- Visual condition data;
- GPS data;
- ROW Video;

2.6.1 Location Referencing Point (LRP) Data

The start and end of each road /link are fixed and defined by PWD, and all objects / assets on these are referred to the start /end of these road /link. However, in circumstances where the start and end points of the road /link are too far apart which may cause cumulative errors in the measurement. In order to reduce these cumulative errors, intermediate reference points called Location Referencing Points (LRPs) shall be identified and recorded during data collection. LRPs are to be located at a position which is deemed permanent in nature, and easily identifiable in the field, generally with maximum intervals of 5km. There would be more prominent structures in urban areas which can be used as LRP. However in rural areas, it is proposed to identify LRPs between 1km (minimum) and 5km (maximum). The following are some of the objects which can be recorded as LRPs during field data collection.

Table 2-2: Location Reference Points

Kilometre Stone	Religious Structure	Permanent Buildings
Cross Drainage Structure	School / College	Intersection

Data Collection: During the survey, the identified LRPs shall be recorded using the ROMDAS software by following the guidelines given in the *ROMDAS User's Guide, Section 8: Location Reference Point Surveys*.



2.6.2 Road Centre Line and GPS Data

As mentioned elsewhere in this document, ROMDAS is equipped with Differential Geographical Positioning System (DGPS), which records the GPS co-ordinates. These co-ordinates can be used to plot the road centrelines with reference to the spatial co-ordinates. The GPS coordinates are recorded automatically by ROMDAS at every 5m intervals and are automatically differentially corrected using the GPS Aided Geo Augmented Navigation (GAGAN) real time data correction stream, and recorded by the ROMDAS software as measured by Trimble SPS 461 receiver. The height of the antenna from the road surface and offset from the centreline shall be input in the ROMDAS software prior to the commencement of the surveys.

Data Collection: Start the ROMDAS software by double clicking on the start icon and follow the below mentioned steps.

- Select Test/Test GPS and confirm the GPS is operating correctly;
- Confirm the GPS is receiving data from sufficient satellites and so giving a valid PDOP reading;
- Start data collection;
- GPS data will be recorded at every 5m intervals.

2.6.3 Video Data- Right of Way (RoW) and Pavement

The video data containing all the assets which are in RoW are automatically be collected using video cameras of the ROMDAS which includes a 3-camera digital imaging system incorporating 1 front, 2 side asset views. The video of the pavement will be captured by 1 rear-camera facing pavement.

Data Collection: As the survey proceeds, ROMDAS video cameras records the videos and stamps *Header data* on the video footage. The *Header Data* includes filename (survey_id), chainage, vehicle speed and the GPS coordinates.

2.6.4 Road Inventory

As a part of road inventory data collection, the details of items such as surface type, width of carriageway, width standard, shoulder type, are collected using the programmable keyboards of ROMDAS. The layout of the labelling of the inventory keyboard is shown in Figure 2-16.



Road Data Collection Procedures

Planning and Road Asset Management Centre (PRAMC)



Figure 2-16: Labelling of Inventory Keyboard

The ROMDAS interface automatically assigns linear chainage and GPS co-ordinates to these inventory items. The inventory data items, method of data collection, and key code configurations are shown in Table 2-3. These configurations can be changed at any point of time in the future, as per then requirement.

Table 2-3: Road Inventory – Key code Configuration

Key code Configuration	Data Item	Key code Labelling
a	Street Light	Street Light Start
d		Street Light End
n	Pavement Edge Marking	Pavement Edge Marking Start
o		Pavement Edge Marking End
PGUP	Overlap	Overlap Start
HOME		Overlap End
x	Median	Median Start
y		Median End
v	Foot Path	Foot Path Start
w		Foot Path End
e	Crash Barrier	Crash Barrier Start
f		Crash Barrier End
l	Centreline Marking	Centreline Marking Start
m		Centreline Marking End
A	Bridge	Bridge Start
B		Bridge End
p	Accident Zone	Accident Zone Start
q		Accident Zone End



Key code Configuration	Data Item	Key code Labelling
I	Pavement Type	Bituminous
J		Concrete
K		Unsealed
E	lane width	3.75
F		5.5
G		7
H		10
P	Shoulder Width	1
Q		1.5
R		2
?	Drain	Lined drain
~		Unlined drain
\		No Drain
T	Shoulder Elevation	Shoulder Drop
S		Levelled Shoulder
U	Shoulder type	Paved Shoulder
V		Gravel Shoulder
Z	Agriculture Land	Agriculture Land
b	Bus Stop	Bus Stop
L	Cautionary Signs	Cautionary Signs
Y	Commercial Land	Commercial Land
0	Forest Land	Forest Land
h	Hectometer stone	Hectometer stone
i	Industrial Land	Industrial Land
8	information sign	information sign
j	Junction	Junction
k	Kilometre Stone	Kilometre Stone
O	Mandatory Signs	Mandatory Signs
C	Minor Bridge	Minor Bridge
W	Mixed Land	Mixed Land
M	Religious Structures	Religious Structures
z	Residential Land	Residential Land
g	Road Boundary Stones	Road Boundary Stones
D	Culvert	Culvert
NUM_-	Educational Institutes	Educational Institutes
s	Speed Breaker	Speed Breaker
t	Tree	Tree
NUM_8	Water Bodies	Water Bodies

Inventory Data Collection: The inventory items observed along the survey route are keyed in by pushing the appropriate key of the configured inventory keyboard. During survey whenever a key on the keyboard is pushed, the distance, GPS coordinates, and the value of the defined parameter are recorded automatically by ROMDAS system interface.



2.6.5 Visual Condition Data

Visual condition data is collected using the semi-automated method (programmable keyboards of ROMDAS and visual observations) at an interval of 500 m. The condition ratings /ranges of various pavement condition data items considered as per the “Guidelines for Maintenance Management of Primary, Secondary and Urban Roads, IRC, 2004” shown in Table 2-4.

Table 2-4: Pavement Defects Ranges – IRC

Defects		Range of Distress			
Cracking (%)	>30	21 to 30	11 to 20	5 to 10	<5
Ravelling (%)	>30	11 to 30	6 to 10	1 to 5	0
Pothole (%)	>1	0.6 to 1.0	0.1 to 0.5	0.10	0
Shoving (%)	>1	0.6 to 1.0	0.1 to 0.5	0.10	0
Patch (%)	>30	16 to 30	6 to 15	2 to 5	<2
Settlement and depression (%)	>5	3 to 5	Up to 2	Up to 1	0
Rutting (mm)	>50	21 to 50	11 to 20	5 to 10	<5
Rating	1	2	3	4	5

The labelling of the visual condition collection keyboard is shown in Figure 2-17. Details of the pavement condition data items along with the ranges which shall be used during the data collection given in Table 2-5 . These configurations can be changed at any point of time in the future, as per then requirement.



Road Data Collection Procedures

Planning and Road Asset Management Centre (PRAMC)

Figure 2-17: Labelling of Visual Condition Keyboard

Data Collection: The surveyor assesses the condition over a 500 m section length and determines the condition value for each of the relevant parameters. The computer provides alert the surveyor by an audible tone to that the assessed condition data to be keyed in. The surveyor presses the appropriate keys and the data to be applied to the respective 500 m section. The data which is keyed in will be locational referenced automatically by ROMDAS interface using the Distance Measuring Instrument (DMI) and Differential Geographic Positioning System (DGPS). Some of the typical examples for collecting the visual condition ratings are presented in Annexure-2.

Table 2-5: Condition Key code Configuration- ROMDAS

Data Item	Key code Labelling	Classification	Key code Configuration
Cracking	Very Good	<5%	CTRL_a
	Fair	5-10%	CTRL_c
	Good	10-20%	CTRL_b
	Poor	20 - 30%	CTRL_d
	Bad	>30%	CTRL_e
Ravelling	Very Good	0%	CTRL_f
	Good	≥1 to 5%	CTRL_g
	Fair	>5 to 10%	CTRL_h
	Poor	>10 to 30%	CTRL_i
	Bad	>30%	CTRL_j
Potholes	Very Good	0 %;	}
	Good	0-0.1 %	
	Fair	0.1- 0.5%;	:
	Poor	0.6- 1.0%;	"
	Bad	>1%	<
Shoving	Very Good	0%	(



Data Item	Key code Labelling	Classification	Key code Configuration
	Good	0.10%	)
	Fair	0.1%-0.5%	-
	Poor	0.6%-1%	+
	Bad	>1%	{
Patching	Very Good	<2%	CTRL_z
	Good	2 to 5 %	%
	Fair	6 to 10%	^
	Poor	11 to 30%	&
	Bad	>30%	*
Depression	Very Good	0	CTRL_k
	Good	up to 1 %	CTRL_l
	Fair	up to 2%	CTRL_m
	Poor	3-5%	CTRL_n
	Bad	>5%	CTRL_o
Bleeding	Very Good	0%	CTRL_p
	Good	5%	CTRL_q
	Fair	5%-10%	CTRL_r
	Poor	10%-30%	CTRL_s
	Bad	>30%	CTRL_t
Edge break	Very Good	0%	CTRL_u
	Good	<5%	CTRL_v
	Fair	5-15%	CTRL_w
	Poor	15-30%	CTRL_x
	Bad	>30%	CTRL_y
Drainage Condition	New		PGDN
	Good		]
	Fair		[
	Poor		,
	Bad		;
Shoulder Condition	Good		!
	Fair		@
	Poor		#
	No Shoulder		\$
Major Vegetation Clearance	Major Vegetation Clearance		-
Minor Vegetation Clearance	Minor Vegetation Clearance		=
No Vegetation Clearance	No Vegetation Clearance		'
Not Motorable	Not Motorable		>
Under Construction	Under Construction		/

2.7 Pavement Roughness and Texture Data

As mentioned elsewhere, Roughness and Texture are measured using two laser profilometers, one each for the left and right wheel paths mounted on the rear side of the vehicle.

Limitations: The following limitations apply for ROMDAS Laser profilometers

- Standoff height (the height from the top of the laser to the ground) is 245mm, and the measuring range is ± 250 mm from this standoff height.



- Minimum survey speed 20 kmph and maximum survey speed up to 100 kmph;
- Not recommended for operation in free standing surface water;
- Data quality is compromised by loose surface material;

Warnings: The Laser DMI key should be removed when the Laser Profilometer is not in use to ensure that it is not operated by untrained personal. The laser is equipped with a mechanical shutter to isolate the laser beam. Ensure the shutter is in the closed position when the Laser Profilometer is left unattended.

Safety Precautions: The exposure to laser radiation emitted from LRMS can be hazardous to the eyes, particularly if the exposure duration exceeds a few seconds while the viewer's eyes are at close proximity of the aperture from which the laser radiation is emitted (worst-case viewing scenario). LRMS system should not be used by anyone who is not aware of these dangers.

Exceptional Situations: The data collection should be paused in some of the exceptional cases

- Road works are underway
- On bridges
- Very low speeds

In such cases, during processing of the data roughness values of the excluded survey sections are estimated by the ROMDAS software, and detailed instructions are given in *ROMDAS User's guide*.

Data Collection: The settings and performance will be checked and tested under the *Test/Test Laser Profilometer* option within the ROMDAS software when required to test the ROMDAS laser IP connections.

- Connect all cables
- Connect the power to the Laser
- Start the laser Connections test
- Start the ROMDAS software data collection program and confirm system operation.

Roughness data to be collected in increasing direction for single lane road and on both increasing and decreasing lanes for two and above lane roads. On four lane roads (divided), the survey vehicle required to be driven on the outermost lane i.e., the lane nearest to the shoulder in each direction.

2.8 Pavement Rut Data

Laser Rut Measurement System (LRMS) is used to collect the pavement rut data.

Data Collection: Rutting data will be collected continuously at the sampling interval set in the LRMS settings. The data will be collected in the increasing direction on single lane road and on both increasing and decreasing lanes on two and above lane roads. On four lane roads (divided),



the survey vehicle will be driven on the outermost lane, i.e., the lane nearest to the shoulder in each direction. The '**Test TPL-LRMS**' option is used to test the ROMDAS LRMS Laser.

- Connect all cables
- Turn on the LRMS key before to start the survey.
- Start the TPL_LRMS test and confirm system operation
- Start the ROMDAS software data collection program and continue survey.

Safety Precautions: The exposure to laser radiation emitted from LRMS can be hazardous to the eyes, particularly if the exposure duration exceeds a few seconds while the viewer's eyes are at close proximity of the aperture from which the laser radiation is emitted (worst-case viewing scenario). LRMS system should not be used by anyone who is not aware of these dangers. More information related to the laser distribution is given in *ROMDAS User Manual* under *Rut Depth Surveys LRMS Scanning Laser* section.

2.9 Daily Checks

Prior to starting the survey, the following checks shall be performed on daily basis

Sr. N ^o	Description	✓	✗
1	Tyre pressure set to standard pressure (35 psi);		
2	Engine oil		
3	Fuel		
4	System power;		
5	Checking all nuts, bolts, connections, cables connected to computer and keyboards;		
6	Cleaning of Lenses of LRMS, Video Cameras with a soft fabric using isopropyl-alcohol or methanol.		
7	Checking ROMDAS Interface and odometer cables;		
8	Testing the operations of , Hardware Interface, GPS, Video Cameras and Lasers		
9	Hose conditions		

Upon completing all the above mentioned checks, survey can be started by switching on the vehicle, power and starting ROMDAS computer.



2.10 Survey Log

During field survey, survey log sheets (Annexure-1) will be maintained by the survey team to record the details of the survey and any other information or factors which may influence the data collection. The details contain the following information:

- Date of survey;
- Road ID
- Chainage (From, To)
- Total Survey Length (km)
- Name of .rbf file
- Any issues occurred during data collection, and countermeasures taken
- Remarks to record any other information such as deviations from the main road, impassable road sections etc.

2.11 General

2.11.1 Survey Direction

The survey direction is by default in the direction of increasing chainage as adopted by PWD and incorporated in the existing RIS database.

2.11.2 Multi lane Roads

On multi-lane roads, ROMDAS survey will be conducted in both increasing and decreasing direction hence it will be necessary to distinguish the increasing and decreasing lane survey files. For this purpose increasing chainage and decreasing chainage lane files will be suffixed with “I” and “D” appropriately.

2.11.3 Naming of Survey Files

All roads will be surveyed link by link with a new file for each link regardless of its length. The file name will be the same as the link number. For example the survey file name of link - BGR001 is BGR001_I for increasing chainage.



3. Falling Weight Deflectometer (FWD)

3.1 General

Pavement strength is assessed by measuring pavement deflection under a pre-determined load using a non-destructive equipment named Falling Weight Deflectometer (FWD). FWD is used to measure the vertical deflection response of a surface to an impulse load using the automated methods. The FWD which is used for data collection under PRAMC is in-vehicle mounted and this equipment comprises of an arrangement to simulate the dynamic load from truck on the pavement. The information collected using FWD can be used to determine (i) Bearing Capacity (ii) Layer Thicknesses (iii) Elasticity Moduli and (iv) Residual life (expected surface life). This FWD machine comprises of the following elements

Table 3-1: Components- FWD Machine

Sr. N°	Component	Purpose
1.	Load Plate - 4 split segmented	For transferring the load on to the pavement
2.	Weight Pack	Various Configurations to apply the desirous loads
3.	Geophone Sensors (9Nos)	For recording the pavement deflections
4.	DMI	Distance Measuring Instrument
5.	Global Positioning System	For recording the spatial coordinates
6.	Temperature Sensors	To measure Ambient, Ground and In Ground temperature
7.	Rugged Laptop	For operating FWD
8.	Load Sensor / Cell (150kN)	To measure the load which is applied
9.	FWD PaveTesting software	to collect the data and create the output files
10.	Controller	to control the FWD Automatic Operations

The FWD equipment is fixed on Tata Xenon 4x4 vehicle and it is in-vehicle mounted as shown in Figure 3-1.



Figure 3-1: FWD- in-vehicle mounted

3.2 Major components of FWD

3.2.1 Load Cell

The load cell fitted to the FWD is designed to measure the load applied to the pavement during the deflection test. Designed with a central bore, the load cell is placed immediately above the loading plate for direct measurement of the loading force (stress). The load cell has a serial number, which is visible from the rear.



Figure 3-2: Load Cell

3.2.2 Load Plate

Directly contacts the pavement surface to transmit the load. The load plate type used by PRAMC is solid, and it has a 300-mm diameter.



Figure 3-3: Load Plate



3.2.3 Geophone Sensors

The geophones of the FWD machine are used to measure the deflection on the pavement surface during the test with an accuracy up to microns. Under PRAMC, the FWD is equipped with 9 numbers of Geophones / Deflection sensors which are mounted at radial distances (in mm) 0, 200, 300, 450, 600, 900, 1200, 1500, and 1800 from the centre of the loading plate (Figure 3-5). Geophones are mounted in spring-loaded sensor support brackets suspended along the sensor bar. Each geophone has a unique serial number that is used to identify critical calibration information in the FWD data collection software. The geophones used in PRAMC FWD are of 2,000-micron.



Figure 3-4: Geophone Sensor

3.2.4 Sensor Bar

It is a long bar mounted above the load plate where the geophone holders are mounted. An extension of the sensor bar continues behind the load plate toward the hitch to allow a geophone to be mounted behind the load plate.



Figure 3-5: Sensor Bar with Geophones

3.2.5 Removable Weights

Removable weights mounted to the weight package. Their mass is 25 kilograms (kg) (44.1 pounds (lb)) each up to 300 Kgs is being used.



Figure 3-6: Removable Weights

3.2.6 FWD Controller

The FWD controller panel is mounted in the left hand side equipment bay and with its processors controls the FWD Automatic Operations. The controller sends the signals to the signal processor located in the vehicle through the multi-signal



cable. Manual Operation can be performed with the use of the controller panel switches mounted on the lid of the controller panel, as per Figure 3-7. However in the current PRAMC FWD, this will be operated through Laptop.



Figure 3-7: FWD Controller

3.2.7 Temperature Sensor – In-ground temperature

FWD is equipped with a temperature sensor for measuring the in-ground temperature (Figure 3-8). With the use of this sensor Sub-surface temperatures shall be measured by drilling 40mm depth holes in the bound layers of the pavement (holes that terminate in an unbound layer may be eliminated) in the centre of the outer wheel path (OWP).



**Figure 3-8:
Sensor for In-
ground
Temperature**

3.2.8 Rugged Laptop

The FWD is equipped with a rugged laptop. This (Figure 3-9) is used to perform automatic tests and to run the Pave Testing FWD software. The Computer is used to input site/test identification data as well as for displaying, storing, editing, sorting and further processing of FWD test data.



Figure 3-9: Rugged Laptop

3.2.9 Data Acquisition Software

FWD is supplied with the PaveTesting FWD Software (Figure 3-10) which is installed on the supplied computer system. This is a standard executable file with additional installation files. This software is used to collect the data and create the output files which are compatible for the necessary further uses.



The output data files created by this software are

- Microsoft CSV for Peak Deflection Data
- Microsoft CSV for Time History Data

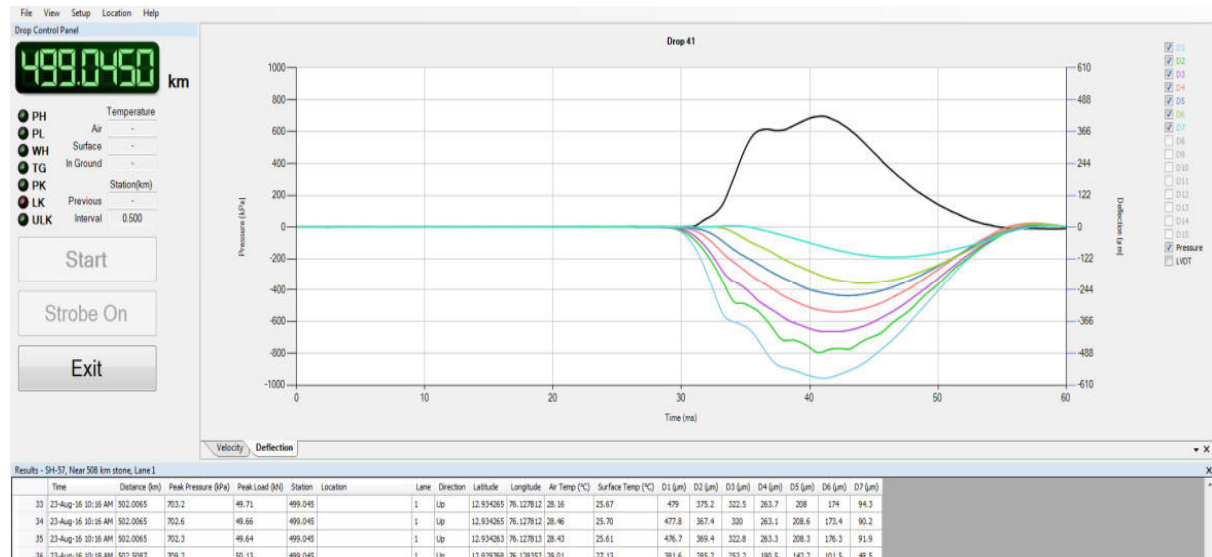


Figure 3-10: Data Acquisition Software- FWD

3.3 Calibration and Verification

3.3.1 Relative Calibration

Relative calibration is a technique used to verify and adjust the response of each of the deflection sensors (geophones) so that equivalent measurements are obtained when the sensors are subjected to the same displacement. Relative calibration involves multiple drops and full rotation of all sensors in all positions in the stand. By rotating the sensors through all positions, any bias due to position in the stand is removed. If a sensor needs to be replaced, the relative calibration procedure can be used to compare it with the remaining sensors to determine a temporary gain factor for the new sensor. Frequent relative calibration of the deflection sensors will ensure that FWD is operating properly. The relative calibration should be performed using a relative calibration stand provided by the FWD manufacturer. It is recommended to contact the vendor whenever if it is deemed relative calibration is necessary i.e., if a major component such as a deflection sensor, multi-signal cable, or trailer PCB board is replaced.

If the FWD is in regular use, relative calibration should be performed on a nominal monthly interval. Relative calibration should not be performed during periods when the FWD is idle. If a major component such as a deflection sensor, multi-signal cable, or trailer PCB board is replaced, the FWD should undergo relative calibration regardless of the interval since the previous relative



calibration. Deflection data should not be collected unless the FWD has undergone relative calibration within the previous 42 days.

3.3.2 DMI Calibration

FWD equipment should undergo DMI calibration monthly while in service. Data should not be collected unless the FWD has undergone DMI calibration within the previous 42 days. If the vehicle undergoes maintenance, including replacement of tires, the DMI should be recalibrated before continuing to collect data, regardless of the interval since the previous DMI calibration.

The FWD-DMI is calibrated by driving the vehicle over a known distance. The FWD data collection software can then calculate an appropriate calibration factor. The section used for DMI calibration must be straight, and maximum of 15 m in length, and reasonably level. Because the FWD must be stopped at each end of the section, it cannot be performed on an active highway without traffic control. The section must be surveyed before the DMI calibration. The section should be measured using a surveyor's tape (a measuring wheel is not acceptable). Ensure that the proper tension and alignment is applied to the tape. Immediately before DMI calibration, the tyre pressure for all of the vehicle tires should be set to the manufacturer's specification (35 psi), and then the operator should drive the vehicle. After each DMI calibration the operator should fill out FWD DMI Calibration form presented in Annexure-3.

3.3.3 Temperature Sensor Verification

Every temperature sensor collecting data associated with measurements should have its accuracy verified monthly, and unless the temperature sensor has been verified during the previous 42 days, data should not be collected. If a temperature sensor fails this procedure, it should be returned to the manufacturer for repair or recalibration, or it should be replaced with a new sensor.

Temperature sensor verification procedure:

- a. Park the FWD and vehicle on a smooth surface in an area with good ventilation that is not exposed to direct sunlight.
- b. Start the FWD data collection software and enter a screen from which the air and Infra-Red (IR) surface temperature measurements can be read.
- c. Unclip the FWD air temperature sensor so that it hangs freely.
- d. Prepare an ice water bath. Place ice and water in the 4 litre bucket and stir with the wooden spoon or paint stirrer. Stir until the reference temperature records a temperature that is less than or equal to 2 °C.
- e. Place the bucket under the FWD air temperature sensor. When the reading from the IR temperature stabilizes, record the temperature from both the IR sensor and the reference



thermometer. Remove the reference thermometer, then stir the ice bath for 1 min, and record the measurements from both sensors again. If the recorded temperatures vary by more than 2 °C for either data set, stir for another minute, and then record both temperatures again. If the IR temperature sensor varies by more than 2 °C from the reference thermometer for two or more data sets, then the IR temperature sensor is unacceptable.

- f. Repeat the procedure in (e) for the hand-held IR temperature sensor. Hold the hand-held IR temperature sensor at a height consistent with the FWD-mounted IR temperature sensor.
- g. Stir the ice bath for another minute. Place the FWD air temperature sensor and the reference thermometer in the bath. When the reading from the air temperature sensor stabilizes, record the readings of both sensors. Stir for another minute and record both temperatures again. If the recorded temperatures vary by more than 2 °C for either data set, stir for another minute, and then record both temperatures again. If the air temperature sensor varies by more than 2 °C from the reference thermometer for two or more data sets, then the air temperature sensor is unacceptable.
- h. Repeat the procedure in (e) for the hand-held temperature probe.
- i. Prepare water bath of the room temperature. Empty the bucket and fill it with the water bath. Allow it to stabilise for 10 min and then stir for 1 min.
- j. Place the bucket under the FWD air temperature mounted sensor. When the reading from the IR temperature sensor stabilises, record the temperature from both the IR sensor and the reference thermometer, Stir the water for 1 min and then record the measurements from both sensors again. If the recorded temperatures vary by more than 2 °C for either data set, stir for another minute and then record both temperatures again. If the IR temperature sensor varies by more than 2 °C from the reference thermometer for two or more data sets, then the IR temperature sensor is unacceptable.
- k. Repeat the procedure in (j) for the hand-held IR temperature sensor. Hold the hand-held IR temperature sensor at a height consistent with the FWD-mounted IR temperature sensor.
- l. Stir the water for another minute. Place the FWD air temperature mounted sensor and the reference thermometer in the water. When the reading from the air temperature sensor stabilizes, record the readings of both sensors. Stir for another minute and record both temperatures again. If the recorded temperatures vary by more than 2 °C (3.6 °F) for either data set, stir for another minute and then record both temperatures again. If the air temperature sensor varies by more than 2 °C (3.6 °F) from the reference thermometer for two or more data sets, then the air temperature sensor is unacceptable.



- m. Repeat the procedure in (l) for the hand-held temperature probe.
- n. If any IR temperature sensor was determined to be unacceptable in the low temperature or ambient temperature check, then it need not be checked at the high temperature. The high temperature check is optional if it is being performed in the field. The high temperature check is to be performed only for IR temperature sensors.
- o. Prepare the high-temperature oil bath. Pour cooking oil into the cooking pot to a depth of approximately 50 mm. Place the cooking pot on the hot plate and under the FWD air temperature mounted sensor. Stir the oil while it is warming on the hot plate. The operator who is stirring the oil must wear gloves. Heat until the oil temperature stabilizes at 60 °C as determined using the FWD IR temperature sensor.
- p. Record the FWD IR temperature sensor and reference thermometer readings. Wait 5 min, and then record both sensors again. If the recorded temperatures vary by more than 2 °C for either data set, then wait 5 min and record both temperatures again. If the IR temperature sensor varies by more than 2 °C from the reference thermometer for two or more data sets, then the IR temperature sensor is unacceptable.
- q. Repeat the procedure in (p) for the hand-held IR temperature sensor. Hold the hand-held IR temperature sensor at a height consistent with the FWD-mounted IR temperature sensor.

3.4 Software Settings and Configuration

3.4.1 Settings

Prior to commencement of FWD surveys, settings related to Application, the Machine and the Drops need to be adjusted as per the requirement. The SETUP section of the FWD system (software) facilitate the operator to modify these settings, the parameters which can be setup under each of these settings are as follows.

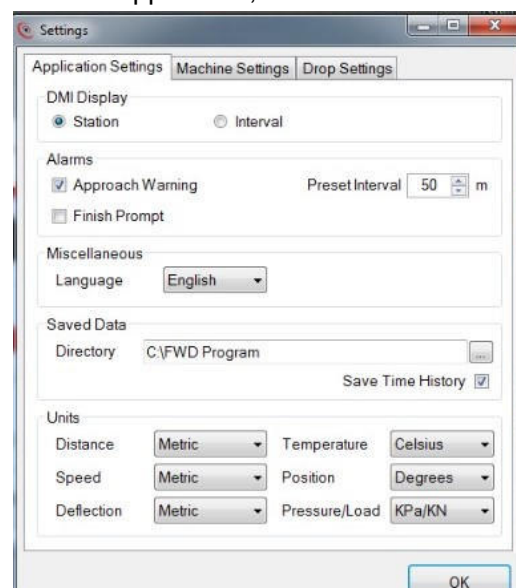


Figure 3-11: FWD Settings



Application Settings: This is used to setup the parameters including DMI Display, Language, Alarms, and the Directory to save the data, and Measurement units. All Measurement units should be in International System (SI) units.

Machine Settings: These settings are used to define the number of Geophone Sensors, sampling time, load plate size and catch pressure.

Drop Settings: A number of drop heights can be pre-programmed in this page allowing quick selection when performing tests. New Drop sequences can be created and deleted. Heights or Pressure can be selected. Max Height not to exceed 300mm.

The above mentioned settings are adjusted to the local conditions and have been input in the FWD software. These settings shall not be altered without any logical reason, as the change in the settings will affect the operation and performance of the FWD equipment. Further detailed instructions on how to enter these settings into the software are given in the *Pave Testing Data Collection Software Manual*.

3.4.2 Configuration of Screen Layout

The FWD test screen layout shall be configured by adding the system which is being used. The screen layout generally includes, Deflection and Velocity Charts, GPS data, and Tabulated results.

3.5 Load Levels

Five load levels are defined for uses under PRAMC. The acceptable load range for each drop height is between 90 percent and 110 percent of the target value. Setting the drop load at 103 percent of the target load at the beginning of the day will minimize the deviation over the course of the day for most cases. Each drop height must be such a way that the targeted load is within the acceptable range. In order to achieve Target Load of 40kN (9kips) (acceptable range 36.0 to 44.0kN (8.1 to 9.9 kips)) the adjustments in the drop height should be performed whenever it is required.

3.6 Preparation for Field Survey

3.6.1 Identification of Homogeneous Sections

FWD data required to be collected on a sample length of 2,500km of Core Road Network (CRN). The representative road sections for conducting FWD survey shall be identified determined according to the IRC 115-2014, by considering the distress condition rutting and cracking the appropriate test plan will be prepared. The crack free test section with rut 0-20mm are considered for the test. This includes the good sections with rutting 0-10 mm and moderate sections with rutting 10-20 mm, while giving due consideration to the geographical conditions, districts



boundaries, soil condition, traffic, connectivity and other factors. The test results of these homogeneous sections will be applied on the remaining sections with similar conditions.

3.6.2 Training survey crew

Prior to proceeding to the field surveys, the survey crew need to be given training in all activities related to equipment operations and a sample survey need to be performed by the team members under the supervision of trainers. This exercise is very much required to assess the understanding levels of the survey team and to provide clarifications on the issues, which they may face during the survey.

3.6.3 Items carried for surveys

The following items shall be carried to the field for collecting FWD

Table 3-2: Items carried for FWD Survey

Sr. No	Description	Remarks
1.	User Manual for GPR and FWD Hard/Soft Copies	For reference and guidance
2.	FWD Complete Equipment Connected with Power Supply unit (Inverter and Battery)	For conducting survey
3.	CRN-Map	Printed Copy
4.	FWD Stretches- List	Printed Copy
5.	Map of FWD Stretches	For reference
6.	FWD Survey Schedule	Printed Copy
7.	FWD Survey Procedure	Respective sections of the Data collection Procedure Report
8.	Log book for recording equipment related issues	For recording the issues in the field
9.	15 Meter Measuring Tape	To measure distance
10.	Road Marking Paint kit with Hand Driven Wheel.	For DMI calibration
11.	Glycerine bottle	To measure in-ground temperature
12.	Drilling Machine Set (Drilling Machine, Battery , Charger, Bits of various Sizes)	For making holes in Pavement to measure temperature
13.	Thermometer	To check In ground Temperature



Sr. No	Description	Remarks
14.	1 Set Walky-Talky hand sets and chargers	For communication between crew members
15.	Traffic Cones	For safety During test
16.	Pens / Pencil	To prepare notes
17.	Note Book / Note Pad	To prepare notes
18.	Highlighters	For marking on the maps
19.	Internet Dongle(With Minimum 3G Capability)	For the Remote Support
20.	Mobile phone with charger	For communication

3.7 Survey Procedure

3.7.1 Interval of Tests

FWD survey will be carried out with the spacing of tests as mentioned below:

- For Double lane roads - 1 kilometre spacing (interval) in each lane/direction (staggered). This spacing is equivalent to 500m between tests and equates to 2 tests per (road) kilometre.
- For Single lane roads testing in one direction only - at 500m spacing. This gives the same numbers of test frequency as for double lanes above.
- For Intermediate roads, where a return journey along the same road is required (for example where this is the shortest connection to the next survey road), testing in both directions at 1 kilometre spacing - as for double lane road. Where it is excessively time consuming to return on the same road, testing to be carried out in one direction, however with staggered tests at 500m spacing on the opposite side of the road.

3.7.2 Deflections Measurement

When an FWD operator arrives at the site, operator should inspect the test section for evidence of maintenance activities.

- Position the FWD so that the centre of the load plate is on the section start limit (i.e., Station 000.000). Set the DMI to 0 centimetres (cm), ascending
- Position the FWD at the first test location
- In the FWD data collection software, set the "Test Setup"/ Road name. Then open a new data file. Use the file name from pre-existing list



- Enter the lane specification appropriate to the test location in the field provided in the data collection software
- Exit the vehicle during testing and examine the pavement surface in the vicinity of the FWD for distresses and defects
- Start the testing sequence while the geophones are lowered to the pavement, the weight is raised and 5 drops conducted at each test point
- The target load of 40kN which corresponds to the load on one dual wheel set of an 80kN standard axle load will be applied and deflection at radial distances (in mm) 0, 200, 300, 450, 600, 900, 1200, 1500, and 1800 from the centre of the loading plate will be measured by using the geophone sensors
- After completing each test point in the pass, close the data file. The data file must be closed and a new data file opened before beginning in a new test pass
- Return the FWD to the section start limit (i.e., station 000+000), and return the DMI to zero
- After testing is complete and the load plate is up, proceed to the next test point

3.7.3 Temperatures Measurement

FWD temperature sensors are used to collect the air, ambient temperature, sub-surface temperatures. Air, and Ambient temperatures are measured automatically by the respective temperature sensors. Sub-surface temperatures shall be measured by drilling 40mm depth holes in the bound layers of the pavement (holes that terminate in an unbound layer may be eliminated) in the centre of the outer wheel path (OWP). The tolerance for holes is ± 10 mm. Allow extra time for the heat generated during drilling to dissipate. If a hole is within ± 25 mm of the reported bottom of the bound layers, its depth should be decreased to 25 mm above the reported bottom of the bound layers. Holes should be drilled with a portable hammer drill using a 13-mm (0.51-inch) diameter bit. After the hole is drilled to the required depth, it should be cleared of debris and dust by blowing through a short piece of plastic tubing or other suitable device. The actual depth of the hole should be measured after it is cleared of debris. Then it should be filled with 15 to 25 mm of mineral oil (glycerine) to provide thermal conductivity between the pavement and the temperature probe. The hole should be covered with tape (such as duct tape) and the tape slit to allow the FWD probe to be inserted.

3.7.4 File Naming

The file name for an FWD data file should be eight characters in the following format: Road Category-Road Number-Km Start Chainage – Km End Chainage. Where



Road Category: SH, MDR; Road Number: 1, 2, 3, 4, 5. etc; Km Start Chainage: 000.000, Km End Chainage: 100.000. For example: Road SH 99 surveyed from Ch: 000.000 to Ch: 100.000 the file Naming will be SH-99-000.000-100.000.

The details of the FWD survey along with the core details are entered in the Form which is shown in Annexure-3.

3.8 Regular Checks

3.8.1 Checks of Vehicle

The operator should perform the following checks on the vehicle before the start of travel for the day:

Regular Vehicle Checks- FWD

Sr. N ^o	Description	✓	✗
1	Fluid levels		
2	Engine oil		
3	Brake fluid		
4	Power steering		
5	Wiper fluid		
6	Coolant		
7	Transmission fluid		
8	Battery connections		
9	Hose conditions		
10	Tires inflated properly and in good condition		
11	Lights operational		
12	Headlights		
13	Taillights		
14	Turn signals		
15	Brake lights		
16	Strobe lights		
17	Interior uncluttered, equipment stowed well		

3.8.2 FWD Checks

The operator should perform the following checks on the FWD before the start of travel for the day:

**Start of Travel Checks- FWD**

Sr. N°	Description	✓	✗
1	Battery connections good.		
2	Transport locks engaged.		
3	Hitch pin in front sensor bar guide.		
4	Covers and latches secure		

3.8.3 Pre-operational Checks FWD

At a minimum, the FWD operator should perform the following checks after the FWD arrives at the test site and before testing begins:

Pre Operational Checks- FWD

Sr. N°	Description	✓	✗
1	Transport locks unlocked.		
2	Hitch pin / Cover removed from front sensor bar guide.		
3	Hydraulic oil level good.		
4	Raise/lower bar cable not frayed and well adjusted.		
5	Geophones well-seated in geophone holders.		
6	Geophone holder springs and foam guides in good condition.		
7	Ribbed rubber sheet on load plate in good condition.		
8	Load plate swivel free-moving.		
9	Pressure switch boots in good condition.		
10	Control box electrical connections tight.		
11	Buffers have no cracks or slits, and are level and tight.		
12	In addition, the operator should observe and listen to the hydraulics during the buffer warm-up sequence to ensure that they are free of air and operating correctly		

3.8.4 Post Operational Checks- FWD

At a minimum, the FWD operator should perform the following checks before leaving the test site:

**Post Operational Checks- FWD**

Sr. N°	Description	✓	✗
1	Transport locks engaged.		
2	Hitch pin/Cover replaced in front sensor bar guide.		
3	Access doors locked.		
4	All supplemental testing equipment properly stowed.		
5	Table's filed out, dated, signed, and filed		
6	Do check all cables are intact/ not damage before starting.		
7	Do check all geophones Holder / Geophones Pins are tight and in place		
8	Do check all Sensors/ Nuts & bolts are tight		
9	Do check if there is any hydraulic oil leakage/ damage hose pipe		
10	Do clean all assemblies daily		
11	Do not keep bags etc. around FWD assembly		
12	Check Hydraulic oil level/if needed top up weekly		
13	In case there is connectivity problem		
	• Check & ensure IP address setting		
	• LAN cable/ connection		
	• Check & re do all Geophones connection		
14	Do not change in any settings in software		
15	On the Geophone beams spare connectors/ socket should be close using cap to avoid any ingress of water or dirt.		
16	At Bridges FWD test should be conducted approx. 50 Meters away from the Approaches of the Bridges and at culverts FWD Testing should be avoided.		

FWD should be maintained in accordance with the manufacturer's recommendations all maintenance and repair activities performed on PRAMC-operated FWDs should be documented using the form shown in Annexure-3.



3.9 Data Checks during Survey

For ensuring good quality data, several checks as mentioned below shall be performed during the survey;

3.9.1 Roll-Off

“Roll-off” is an error condition that results when the deflection of the pavement surface as recorded by a deflection sensor does not return to near zero within 60 ms of the trigger activation. Unless the pavement structure is weak enough to be permanently deformed by the load pulse, this is not a believable measurement.

Roll-off can be caused by poor contact between the deflection sensor and the pavement surface. This error can be incorrectly triggered when magnitude of the deflection approaches the resolution of the geophone. Thus, roll-off when the peak deflection is less than 25 μm is not necessarily indicative of error.

If the roll-off error is triggered on a deflection that is greater than 25 μm , then the operator should follow the error resolution instructions in *section 3.9.6*. Otherwise the measurement should be accepted and testing should continue.

3.9.2 Non-Decreasing Deflections

“Non-decreasing deflections” is an error condition that results when the deflections measured by the deflection sensors do not decrease with increasing distance from the load plate. Deflections should always decrease with increasing distance from the load plate.

This condition can sometimes occur legitimately if there is a transverse crack or other discontinuity between the two sensors that exhibit the non-decreasing deflections. If the operator observes such a crack or discontinuity between the two flagged sensors, then the test should be accepted and the observation should be recorded at the “Comments” prompt.

Non-decreasing deflections can sometimes be triggered incorrectly when the magnitude of the deflections approaches the resolution of the deflection sensor. If the larger of the two deflections is less than 10 μm , then the test should be accepted, and testing should continue.

Non-decreasing deflection may sometimes occur between deflection sensors 1 and 2 on extremely weak pavements because of permanent deformation of the pavement by the FWD.

If the deflections are not small and there are no cracks or discontinuities, or the non-decreasing deflections are not between sensors 1 and 2 and the pavement is not very weak or very stiff, then this error usually indicates poor seating between one or both of the flagged sensors and the pavement surface. The operator should follow the error resolution instructions given in *section 3.9.7*.



3.9.3 Overflow

“Overflow” is an error condition that results when a measured deflection exceeds the range of the deflection sensor. For the FWDs operated by PRAMC, this range is 2,000 microns (80 mils, or 0.08 inch). Deflections of that magnitude are only expected on extremely weak pavements or when testing using extremely high load levels. For testing at a PRAMC test section using the load levels specified in this document, it is not reasonable to expect that the deflections will exceed 2,000 microns. If this error is encountered during testing, it is likely that the flagged sensor does not have good contact with the pavement surface. The operator should follow the error resolution instructions given in *section 3.9.6*.

3.9.4 Load Variation

“Load variation” is an error condition that results when the peak load for repeat drops at the same drop height varies by more than the amount specified in *section 3.5*. This condition can occur legitimately on weak pavements where the structure is damaged by FWD testing, or on pavements with a saturated base or subgrade layer (such as during the spring thaw in wet-freeze zones). It may also occur if the load plate is not seated properly on the pavement surface, either because of loose debris or irregularities in the pavement surface.

If this error occurs during testing, the operator should follow the error resolution instructions given in *section 3.9.7*.

3.9.5 Deflection Variation

“Deflection variation” is an error condition that results when the load-normalized peak deflections for repeat drops vary by more than the amount specified in *section 3.5*. This condition can occur legitimately only if the stiffness of the pavement is changed by the FWD testing itself. This generally occurs only if the pavement is extremely weak or the unbound layers are saturated.

Deflection variations can occur as a result of uneven pavement surface conditions causing poor seating of the load plate or deflection sensors or vibrations generated by heavy equipment operating nearby, especially trucks traveling in adjacent lanes.

If this error occurs during testing, the operator should follow the error resolution instructions in *section 3.9.6*.

3.9.6 Dealing with Deflection Errors

If deflection errors occur, the operator must attempt to identify the source of those errors. If the errors caused by the FWD equipment, then those problems must be fixed before testing continues. If the errors result from localized pavement conditions, the operator should reposition the FWD and comment on the condition. If the errors are the result of pavement conditions that



are representative of the test section as a whole, or due to factors beyond the operator's control such as truck traffic, the operator should accept the error and comment on the condition.

Sections 3.9.1, 3.9.2, 3.9.3, and 3.9.5 detail possible causes of the various types of deflection errors. If the deflection errors appear to be to the result of truck traffic in an adjacent lane, the operator should attempt to pause the test sequence to allow the trucks to pass, and then continue the sequence during lulls in the traffic. If there are too many trucks for this method to be practical, the operator should provide a comment for drop sets that were potentially affected by the truck traffic.

Operators must be very careful to not overlook the FWD equipment itself as the source of the error. The following list is a summary of the troubleshooting process that is recommended in all cases, and the operator should deviate from the process only when confident of the source of the error, and the error source is not well suited for resolution using this troubleshooting process.

- a) For the first such error at a test location, it is recommended that the operator get out of the vehicle and check the flagged deflection sensor (or sensors). Check that the deflection sensor is seated securely in the sensor holder, that the screws retaining the sensor magnet and sensor holder are tight, that the deflection holder is not resting on a loose stone or crack, and that the holder springs and foam bushing are in good shape.
- b) If several sensors are flagged, it is recommended that the operator check all analog connections: sensor to control box, control box to multi-signal cable and multi-signal cable to signal processor.
- c) The data should be rejected and the test repeated without repositioning the FWD.
- d) If the errors persist, the operator must reject the test and perform the optional activities in mentioned in sections (a) and (b). If problems with the equipment are discovered and corrected, all data collected in this test pass should be discarded, and the test pass should be restarted.
- e) If errors persist and the test being performed is a load transfer test, or if the FWD cannot be moved forward because of a joint or transverse crack, then the results should be accepted whether or not they contain errors. The operator should enter a comment stating "Error could not be resolved."
- f) If the errors persist and the FWD can be repositioned, the operator should reject the data and move the FWD forward by 0.6 meters. The operator should repeat the test a fourth time. If the errors persist the operator should accept the results and at the prompt enter a comment stating "Error could not be resolved."
- g) If the error could not be cleared, then for all subsequent errors of the same type in the same test pass the activities listed in sections (a) and (b) need not be repeated.



3.9.7 Dealing with Load Errors

If load errors occur, the operator must attempt to identify the source of those errors. If the errors result from problems with the FWD equipment, then those problems must be fixed before testing continues. If the errors are caused by localized pavement conditions, the operator should reposition the FWD and comment on the condition. If the errors are caused by pavement conditions that are representative of the test section as a whole, the operator should accept the error and comment on the condition. *Section 3.9.4* gives details of the possible causes of load errors.

Operators must be very careful to not overlook the FWD equipment itself as the source of the error. The following summary of the troubleshooting process is recommended in all cases, and the operator should deviate from it only if confident of the source of the error and the error source is not well suited for resolution using the troubleshooting process.

- a) The data should be rejected and the test repeated without repositioning the FWD.
- b) If the error persists, the operator should get out of the vehicle and check the equipment. All analog connections should be checked: load cell to load cell cable, load cell cable to control box, control box to multi-signal cable and multi-signal cable to signal processor. The weight height targets should be checked to ensure that they are tight. The load plate should be raised and the swivel checked to ensure that it moves easily. Check the rubber sheet and pavement surface under the load plate for debris, and remove any.
- c) Reject the data and repeat the test without repositioning the FWD.
- d) If errors persist and the test being performed is a load transfer test, or if the FWD cannot be moved forward because of a joint or transverse crack, then accept the results whether or not they contain errors. Enter a comment stating “Error could not be resolved.”
- e) If the errors persist and the FWD can be repositioned, reject the data and move forward the FWD by 0.6m. Repeat the test a third time. If the errors persist then accept the results, and enter a comment stating “Error could not be resolved.”
- f) If the error could not be cleared, then for all subsequent errors of the same type in the same test pass, the operator need not repeat the activities in step (b).

3.10 Corrections of Collected Data

- The data collected, deflections should be in decreasing order from geophone D1 TO D9.
- If any of the deflection from the sensor D1 to D9 is missing / more than it's precede geophone, such values should be corrected with the average reading of the test data from the remaining Drop tests conducted at the same location.



- The first row of Deflections should not be blank. In case of any missing in data the complete drop test is omitted or the immediate drop in the same location is to be used for the back calculation.
- If the First Geophone Data/ majority of the Geophones data collected is invalid/Missing for all the drops at a Location. Such data should not be processed further.

3.11 Test Results – FWD

The data acquired by the FWD software can be exported to .csv format. A sample FWD test report containing the information of, geophone spacing, location referencing details, Drop ID, height of drop, applied pressure, deflections recorded by various geophone sensors, is shown in Table 3-3.



Road Data Collection Procedures

Planning and Road Asset Management Centre (PRAMC)

Table 3-3: Results Table- FWD

1	Available Sensors	9																						
2		D01	D02	D03	D04	D05	D06	D07	D08	D09														
3	Sensor Offset X	0	200	300	450	600	900	1200	1500	1800														
4	Sensor Offset Y	0	0	0	0	0	0	0	0	0														
5	Load plate size	300	mm																					
6	Time	Location	Up/Down	Lane	Station (km)	Latitude	Longitude	DropID	Requested Height (mm)	Pressure (kPa)	Load (kN)	Air Temperature (°C)	Surface Temperature (°C)	In Ground (°C)	D1 (µm)	D2 (µm)	D3 (µm)	D4 (µm)	D5 (µm)	D6 (µm)	D7 (µm)	D8 (µm)	D9 (µm)	Remark
7	19-04-2016 15:42	palanahalli	Up	1	155.75	13.44244	77.23636	1	129.8	601.11	42.49	40.37	29.14	42.3	706.85	484.37	311.12	224.6	132.2	102.59	72.42	65.39	34.58	
8	19-04-2016 15:42	palanahalli	Up	1	155.75	13.44244	77.23636	2	130.49	606.27	42.85	40.76	34.85	42.18	702.14	462.36	307.91	224.08	137.16	99.66	74.33	65.44	40.43	
9	19-04-2016 15:42	palanahalli	Up	1	155.75	13.44244	77.23636	3	130.26	611.07	43.19	40.52	32.81	42.09	696.27	460.41	307.58	223.71	139.2	102.72	73.52	67.92	27.16	
10	19-04-2016 15:42	palanahalli	Up	1	155.75	13.44244	77.23636	4	130.72	610	43.12	40.52	35.26	42.72	692.83	464.85	303.86	223.18	138.13	101.48	76.65	63.94	26.44	
11	19-04-2016 15:42	palanahalli	Up	1	155.75	13.44244	77.23636	5	130.03	609.81	43.1	40.49	28.9	42.21	689.06	471.23	301.85	223.46	140.16	103.1	74.81	66.47	24.66	
12	19-04-2016 15:45		Up	1	156.25	13.44689	77.23648	6	130.15	605.61	42.81	40.43	27.55	42.48	893.05	641.91	420.74	310.38	199.74	146.04	109.17	84.84	50.04	
13	19-04-2016 15:45		Up	1	156.25	13.4469	77.23647	7	130.26	611.16	43.2	40.7	29.08	42.51	889.98	640.02	422.8	314.46	201.8	148.92	109.98	88.24	50.04	
14	19-04-2016 15:45		Up	1	156.25	13.4469	77.23647	8	130.15	609.75	43.1	40.82	32.48	42.42	884.66	636.91	420.1	313.91	202.99	151.19	109.72	88.73	50.04	
15	19-04-2016 15:45		Up	1	156.25	13.4469	77.23647	9	130.15	610.41	43.15	40.94	22.76	42.45	880.8	640.22	417.71	312.71	203.2	148.54	111.94	86.44	50.04	
16	19-04-2016 15:45		Up	1	156.25	13.4469	77.23646	10	130.72	612.33	43.28	41.12	30.03	42.48	879.97	639.34	422.33	313.43	203.1	147.47	112.51	89.9	50.04	
17	19-04-2016 15:47		Up	1	156.75	13.45148	77.23658	11	129.69	616.65	43.59	40.52	20.71	42.42	555.83	395.75	299.03	237.27	168.7	112.92	80.46	58.92	Invalid	
18	19-04-2016 15:48		Up	1	156.75	13.45148	77.23658	12	130.6	619.05	43.76	40.88	25.75	42.45	555.32	393.48	295.7	238.03	166.29	111.26	80.6	60.32	Invalid	
19	19-04-2016 15:48		Up	1	156.75	13.45148	77.23658	13	130.15	619.77	43.81	40.91	21.85	42.57	552.07	404.36	295.2	238.39	167.98	112.78	80.24	61.33	4.56	
20	19-04-2016 15:48		Up	1	156.75	13.45147	77.23658	14	130.26	619.77	43.81	41.24	24.26	42.57	554.71	399.69	294.12	237.35	164.73	111.6	83.38	59.81	10.18	
21	19-04-2016 15:48		Up	1	156.75	13.45146	77.23658	15	130.72	623.13	44.05	41.73	29.76	42.66	557.96	402.59	295.69	239.63	165.43	113.01	84.34	61.11	0.04	
22	19-04-2016 15:50		Up	1	157.24	13.45583	77.23548	16	130.49	614.84	43.46	40.88	26.95	42.63	841.34	590.23	391.97	289.26	171.89	112	77.28	61.71	47.42	
23	19-04-2016 15:50		Up	1	157.24	13.45585	77.2355	17	129.92	611.55	43.23	41.03	36.32	42.51	814.16	569.76	388.76	281.29	171.96	103.67	79.7	65.88	45.35	
24	19-04-2016 15:50		Up	1	157.24	13.45585	77.23549	18	130.26	612.36	43.29	41.36	25.65	42.57	813.38	567.57	384.84	282.56	173.53	102.57	79.9	64.84	46.04	
25	19-04-2016 15:50		Up	1	157.24	13.45585	77.23549	19	129.8	615.06	43.48	41.39	24.99	42.63	817.12	576.32	384.9	280.27	171.89	109.66	80.32	66.05	44.03	
26	19-04-2016 15:51		Up	1	157.24	13.45584	77.23549	20	129.58	611.54	43.23	41.51	26.59	42.69	816.8	579.84	382.64	286.69	172.59	106.39	80.36	66.32	43.39	
27	19-04-2016 15:53		Up	1	157.73	13.46007	77.2341	21	130.6	605.88	42.83	41.21	27.98	42.81	660.37	442.78	291.97	204.86	113.82	74.82	60.53	52.71	40	
28	19-04-2016 15:53		Up	1	157.73	13.46007	77.23409	22	130.49	606.24	42.85	41.48	28.48	42.96	645.5	449.07	291.43	203.92	113.68	73.29	61.71	55.08	40.11	
29	19-04-2016 15:53		Up	1	157.73	13.46006	77.23409	23	130.72	610.27	43.14	41.6	29.17	42.54	647.93	431.97	290.25	203.4	115.96	73.26	67.58	54.35	39.45	

SH-003 2016-04-21



4. Ground Penetrating Radar (GPR)

4.1 General

Ground Penetrating Radar (GPR) is a geophysical method that uses radar pulses to image the subsurface so that it can be used for identification of the pavement layers thicknesses. GPR is non-destructive equipment and provide layers internal details without the need for breaking the pavement. The GPR system of PRAMC provide complete assessment of road conditions, such as:

- Pavement thickness measurement;
- Surface, base and sub-base road course assessment;
- Detection of cavities, voids and delamination.

GPR is mounted on the front side of vehicle in which FWD is in-vehicle mounted. The GPR system which is used under PRAMC is shown in Figure 4-1.



Figure 4-1: GPR setup on Vehicle

The GPR system comprises of the following components

Sr. N°	Component	Purpose
1.	Distance Measurement Instrument (DMI)	a distance measurement device
2.	Global Positioning System	For recording the spatial coordinates



3.	Horn Antennas	For detecting the depth and alignment of layers
4.	Digital Antenna Driver(DAD) Controller	to control the operation of antennas and record the DMI(encoder) readings
5.	Rugged Laptop	For operating GPR through DAD controller

4.1.1 Digital Antenna Driver (DAD) Controller

The Control unit is the brain centre for the GPR system and is responsible for coordinating the operation of the other components of GPR. It is the control unit responsible for directing the antennas and digitalising the acquired radar data.



Figure 4-2: DAD Controller

4.1.2 Horn Antennas and Dual Frequency Antenna

GPR collects the data using 4-channel system with each channel is having one frequency and these frequencies are generated by separate antennas but all antennas are controlled by single control unit in the vehicle. Antennas does the work of transmitting radar signals and receiving the reflected waves. The frequency of the antennas used are 2GHz, 1GHz, 900MHz and 400MHz (Figure 4-3 and Figure 4-4).



Figure 4-3: Horn Antenna



Figure 4-4: Dual Frequency Antenna



4.1.3 Distance Measurement Instrument (DMI)

The Encoder is generally a mechanical device, such as a wheel, that is used to measure distance along the target area and initiates a triggering pulse for the radar signal at predetermined distances. In some cases, a GPS is used in the Encoder to determine location and distance for triggering.



Figure 4-5: DMI

4.1.4 Ratchet Belts

The height of the antennas are adjusted by tying the fibre glass frame to the vehicle body with the ratchet belts. These belts are non-metallic in nature, and it is mandatory to use only non-metallic belts, because if metal belts or other ropes are used those metallic particles /body will interfere with the radio waves of the GPR Antennas which in turn will affect the quality of the information acquired.



Figure 4-6: Ratchet Belts

4.1.5 Data Acquisition Software

K2-Fastwave acquisition Software is used for collecting the data in the field. This software is installed on a Notebook Computer which is supplied along with the GPR system.



Figure 4-7: Data Acquisition Software-Start up Screen

4.1.6 Rugged Laptop

The K2-Fastwave acquisition software is installed on a Notebook Computer, which is dedicated to the specific phases of setting up, acquiring and saving radar data. The data logger is used for controlling the operations of GPR, to acquire and store the data collected in the field. The data acquired through will be further processed using the GRED HD software.



Figure 4-8: Rugged Laptop

The configuration of the data logger used in GPR survey is:

- Operative system: Windows 7
- Processor: Intel i5 mobile tech
- RAM: 4GB Ram
- Hard Disk: 500 GB HDD, shock-damped
- Lan: 100 Base-TX/10 BASE

Note: No communication software of the type Firewall, Wi-Fi or Antivirus protection may be installed on the computer; these types of software enter into conflict with the acquisition software.

4.2 Setup of Acquisition Software

The K2_FW acquisition software is used to manage the phases of radar acquisition and to review the data acquired directly on site. The DAD Fast Wave has an “Interlaced” acquisition mode that



allows the acquisition of double channels almost simultaneously, this permits an acquisition velocity twice as fast as normal K2_FW systems.

The operative phases of the K2_FW acquisition software occur in the following sequence:

- Start up the K2_FW software
- Choose the configuration
- Set up the calibration
- Choose the survey
- Set the acquisition parameters
- Acquire the radar data
- Review the data

Please refer to *GPR user manual* for more details about the above mentioned phases.

4.3 Daily Checks

Check the functional status of the system with the reference of spy lights representing the radar (RADAR), battery (BATT), and acquisition speed (SPEED) and acquired sweeps (SWEEP). The system function spy lights

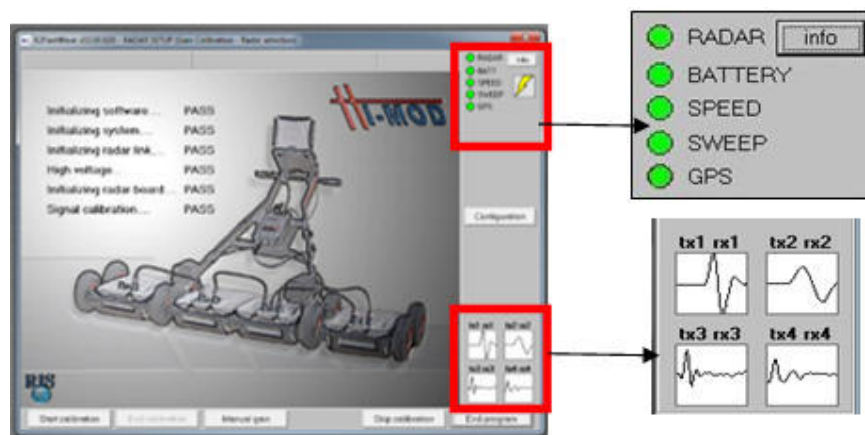


Figure 4-9: Spy lights

When the spy lights are coloured

- White: this means the system is not active
- Green: this means the system is functioning correctly
- Red: this means the system is not functioning correctly

Check all the antennas connected in the order and are working, any antenna displaying the straight line instead of wave form needs the checking of connections

- The list of tests to be pass as shown in Figure 4-9, if any fail in the tests please check the connections



- Order of the antenna positions which are to be connected
 - 400 MHz: TX1 RX1,
 - 900 MHz: TX2 RX2,
 - 2000 MHz: TX3 RX3,
 - 1000 MHz: TX4 RX4

4.4 Survey Procedure-GPR

4.4.1 Calibration

This paragraph describes the procedure used to calibrate the parameters used in the real-time processing of radar data.

- Press the Start Calibration button shown in Figure 4-9 to view the window shown in Figure 4-10, then perform the automatic calibration procedure by moving the antennas across the surface to be scanned for a distance of at least one meter.

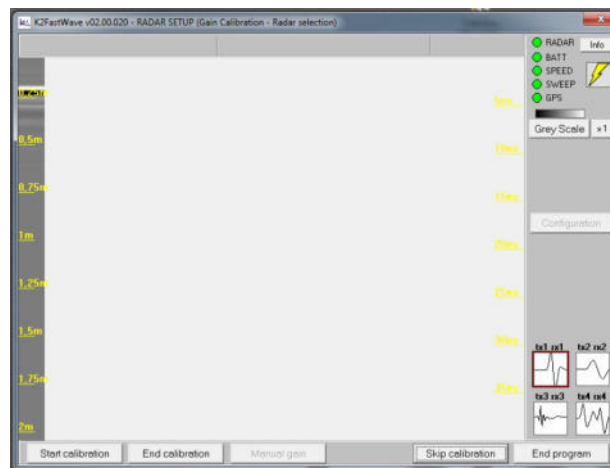


Figure 4-10: Calibration is underway

- Once the Calibration Gain is completed, Survey Details window will appear Automatically as shown in Figure 4-11, the Naming is to be given for the survey and click on **NEW ACQUISITION**
- For Resuming the acquisition selected road from the list of surveys appearing and click on **RESUME ACQUISITION** as shown in Figure 4-11.

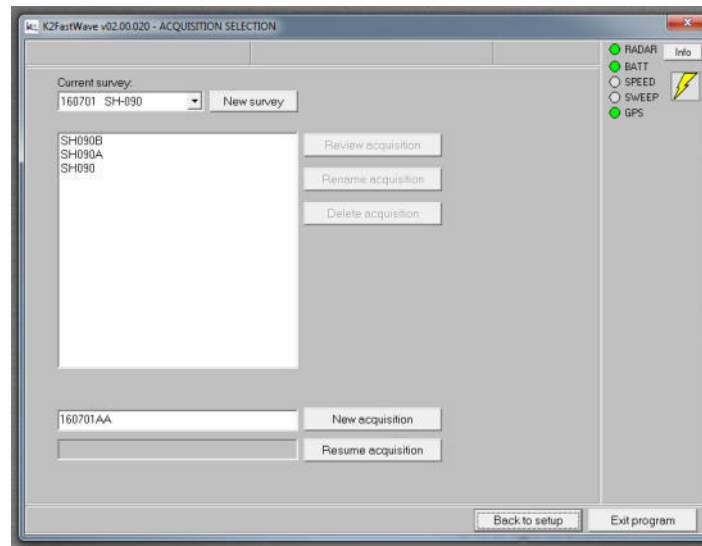


Figure 4-11: Acquisition Selection window

- After completion of the survey click on end acquisition which automatically saves the collected data as shown Figure 4-12 and stored in the Directory **C:\K2\MISSIONS** (with the file name the data acquired).

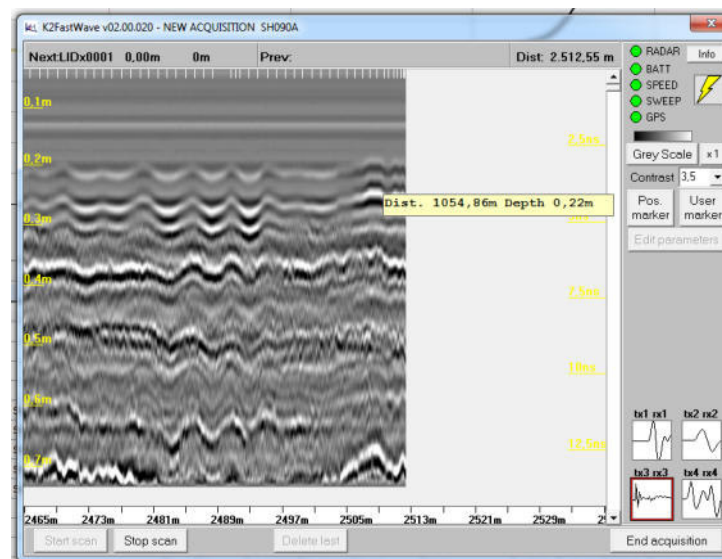


Figure 4-12: Acquired Data

4.4.2 Sampling Interval

GPR is an automatic, continuous test that gives the details of the pavement layers. The pavement layers details at the point of deflection measurements will be collected. However, as GPR is an automatic and continuous test, the pavement layers details of the road sections irrespective of the FWD intervals will be captured for the entire length of the homogeneous sections.



4.4.3 Procedure of Data Collection

GPR collects the data using 4-channel system with each channel is having one frequency and these frequencies are generated by separate antennas but all antennas are controlled by single control unit in the vehicle. The frequency of the antennas used are 2GHz, 1GHz, 900MHz and 400MHz. The depth of penetration of the waves in to the ground is inversely proportional to the frequency of the antenna. Using the GPR, the pavement depth up to 1.5m can be covered. GPR referencing will be done by either GPS and/or by DMI (chainage) and the analysis software will provide the results in the spreadsheet which comes along with the GPR software. The GPR analysis of the data is typically interactive and semi-automatic.

4.5 Test Pit

Upon collecting the field data the GPR results are interpreted with reference to the control locations (where the asphalt thickness and pavement thickness details are known). The existing pavement layers and thicknesses information will be collected through secondary sources of PWD records of maintenance history. In the absence of such secondary information, the existing pavement layers information will be collected through primary surveys by excavating test pits.

4.5.1 Sampling Interval

Test pits will be dug 1 in every homogeneous section. The length of the homogeneous section vary depending the inventory and condition and other characteristics of the road.

4.5.2 Data Collection

A test pit of 0.6mX0.6m will be excavated at the outer edge of the outer lane of the pavement in the earthen shoulders exposing pavement layers sufficiently to note down the pavement layer thickness and pavement material type. After collecting the necessary details from the test pit, it will be filled back with suitable materials and compacted properly so that it will not create any safety hazard to the traffic. At each test pit, the number of layers observed, thicknesses of the layers, pavement material type, and location of the test pit will be recorded.

4.6 Items carried for Surveys

For performing the GPR test and to dig the test pits, the following items will be carried to the field.

Table 4-1: Items carried for GPR and Test Pit Surveys

Sr. No	Description	Remarks
1.	GPR Equipment	For collecting pavement layers information
2.	Data Logger (Laptop)	To collect the data in the field, operation of GPR, processing of the data



Sr. No	Description	Remarks
3.	Shovel	For Excavating test pit
4.	Crowbar	For Excavating test pit
5.	Mechanical Kit	To mount Fibre glass beams
6.	Digital Camera & Charger	to collect photos of test pits
7.	Internet Dongle(With Minimum 3G Capability)	for the Remote Support
8.	Mobile phone with charger	For communication
9.	Pens / Pencil	For recording the remarks
10.	Note Book / Note Pad	To record the survey status
11.	Highlighters	For marking on the maps

4.7 Analysis of Data

After the data collection the data will be analysed to assess the pavement layers, composition, and thicknesses. The procedure of analysing the GPR data is provided in the *GPR Manual*.

**Annexure1
Survey Log - ROMDAS**

Survey Date	Road ID	Kilometerage		Survey Length (km)	Name of .rbf file	Equipment Related			Remarks/ Observation	Signature of Operator
		From	To			Issues	Possible Causes	Issue Resolution		



Annexure2

ROMDAS

Typical images for visual Condition Rating

Cracking:

Cracking will be measured in percent of surface area.



Very Good (<5%)



Good (5-10%)



Fair (11-20%)



Poor (21-30%)



Bad (>30%)



Ravelling:

Ravelling will be measured in percent of surface area.



Very Good (0%)



Good (1-5%)



Fair (6-10%)



Poor (11-30%)



Bad (>30%)



Potholes:

Potholes will be measured in percent of surface area.



Very Good (0%)



Good (0-0.1%)



Fair (0.1-0.5%)



Poor (0.6-1%)



Bad (>1%)



Patching:

Patching will be measured in percent of surface area.



Very Good (<2%)



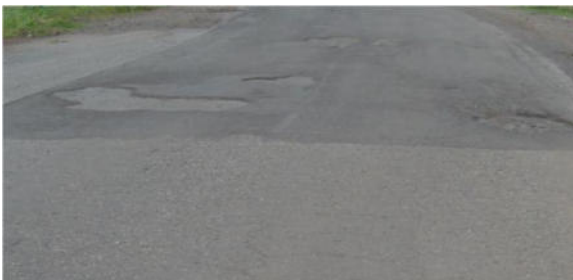
Good (2-5%)



Fair (6-15%)



Poor (16-30%)



Bad (>30%)



Depressions:

Depressions will be measured in percent of surface area.



Very Good (0%)



Good (0-1%)



Fair (1-2%)



Poor (3-5%)



Bad (>5%)



Bleeding:

Bleeding will be measured in percent of surface area.



Very Good (0%)



Good (<5%)



Fair (5-10%)



Poor (10-30%)



Bad (>30%)



Road Data Collection Procedures

Planning and Road Asset Management Centre (PRAMC)

Annexure3

(FWD)

Survey Log Details - FWD

S. N o	Date	Road No	Survey File Name	Road Name	Chainage in Km		Length Surveyed in Km	Direction of Survey	Temperature (In Ground) in Centigrade	Temperature @ Chainage in km	Core Data Collected Chainage	Core Details				Remarks
					From	To						Layer 1	Lay er 2	Layer 3	Layer 4	



FWD machine Maintenance and Repair Summary

Date	Odometer	Problem Description	Description of Maintenance	Performed By	Cost		
					Labour	Parts	Total



FWD DMI Calibration

Date (DD-MMM-YYYY) ____ ____ - ____ ____ - ____ ____ ____

Section Length (feet) ____ ____ ____ ____

New Calibration Factor
(Counts per km) ____ ____ ____, ____ ____ ____

Performed By: _____



Government of Karnataka

Public Works Department

ROAD DATA PROCESSING AND QUALITY ASSURANCE PROCEDURES



TABLE OF CONTENTS

<u>1. INTRODUCTION.....</u>	<u>1</u>
1.1 INTRODUCTION	1
1.2 SCOPE OF THIS DOCUMENT.....	1
<u>2. DATA PROCESSING- ROMDAS</u>	<u>2</u>
2.1 SURVEY DATA SUBMISSION	2
2.2 DATA RECEIVING	2
2.2.1 FILE STRUCTURE	2
2.2.2 FILE NAMING CONVENTION	3
2.3 PROCESSING OF RAW DATA.....	3
2.4 VALIDATION OF PROCESSED DATA	5
2.5 DATA COMPILATION.....	5
<u>3. DATA PROCESSING- FALLING WEIGHT DEFLECTOMETER (FWD)</u>	<u>6</u>
3.1 GENERAL	6
3.2 THEORETICAL BACKGROUND	6
3.3 VALIDATION OF THE ACQUIRED DATA	7
3.4 INSTALLATION OF DAPS SOFTWARE	8
3.5 PROGRAM USE.....	8
3.6 CONSTRUCTION DATA.....	9
3.7 VIEW FWD BOWLS.....	11
3.8 PROCESSING OF FWD DATA.....	13
3.8.1 OPTIONS.....	17
<u>4. DATA PROCESSING -GROUND PENETRATING RADAR (GPR)</u>	<u>20</u>
4.1 GENERAL	20
4.2 SYSTEM REQUIREMENTS	20
4.3 SOFTWARE MODULE SKETCH.....	20
4.4 FILE MENU.....	21
4.4.1 OPEN PROJECT.....	21
4.4.2 DATA LOADING AND POSITIONING	22
4.4.3 SETTINGS- PROCESSING THE LAYERS.....	27
4.4.4 CALIBRATION OF THE PLOTTED LAYERS	30
4.4.5 RESULTS GENERATION	31



4.5 APPLICATION OF GPR AND FWD RESULTS..... 33

LIST OF TABLES

Table 2-1: Data receiving Log file.....	2
Table 3-1: .BLS file columns Description.....	14
Table 3-2: Results obtained in “b/s” format.....	16
Table 4-1: KRAMS Format- FWD and GPR Results	34

LIST OF FIGURES

Figure 2-1: File Structure	3
Figure 2-2: Selecting <i>Data Processing</i> Option- ROMDAS	3
Figure 2-3: Selecting raw file for processing.....	3
Figure 2-4: Adding files for Processing.....	4
Figure 2-5: Data Processing underway	4
Figure 2-6: Selecting applicable data for processing	4
Figure 2-7: Microsoft Access (.mdb) files created.....	5
Figure 3-1: Working Directory Notes- DAPS	9
Figure 3-2: Example of Pavement Construction Types.....	10
Figure 3-3: Saving Construction Data	11
Figure 3-4: Option <i>View FWD Bowls</i>	11
Figure 3-5: Indication of Construction File completion.....	12
Figure 3-6: Indication Graph Showing Load/Deflection.....	13
Figure 3-7: Back Calculation Results	14
Figure 3-8: Layer 1 & Half Space Parameters.....	18
Figure 3-9: Layer 1 & Half Space Parameters.....	19
Figure 3-10: Transducer spacing and load-plate diameter	19
Figure 4-1: File Menu- GRED HD software	21
Figure 4-2: Project Selection window	22
Figure 4-3: Load Mode Selection window.....	22
Figure 4-4: Geographical reference module.....	23
Figure 4-5: Processing of Geographical Reference Module without GPS coordinates.....	24



Figure 4-6: Navigation window with reference scans.....	25
Figure 4-7: Project Explorer Tree	26
Figure 4-8: Window Showing the Scanned Layers	26
Figure 4-9: Process Setting Menu	28
Figure 4-10: Process Setting Window	29
Figure 4-11: Processed Layers Window after the Application of the Required Setting.....	29
Figure 4-12: Plotting the Pavement Layers	30
Figure 4-13: Inserting the core Details	31
Figure 4-14: Layer Legend	31
Figure 4-15: Saving the Plotted Layers	31
Figure 4-16: Exporting the Layers to Ms Excel.....	32
Figure 4-17: Exported Layers Thickness	32
Figure 4-18: Corrected Layers Thickness.....	32



ABBREVIATIONS

CDT	Construction data
CRN	Core Road Network
DAD	Digital Antenna Driver Controller
DAPS	Deflection Analysis of Pavement Structures
FWD	Falling Weight Deflectometer
GPR	Ground Penetrating Radar
GPS	Geographical Positioning System
GRED HD	Georadar Data Elaboration High Density
KRAMS	Karnataka Road Asset Management System
MDR	Major District Road
MHz	Megahertz
NH	National Highways
PRAMC	Planning and Road Asset Management Centre
PWD	Public Works Department
RMS_{err}	Root Mean Square error
ROMDAS	ROad Measurement Data Acquisition System
SH	State Highway
USB	Universal Serial Bus
UTC	Coordinated Universal Time



1. Introduction

1.1 Introduction

Planning and Road Asset Management Centre (PRAMC) is responsible for determining the annual maintenance requirements of the road network under jurisdiction of PWD. Currently PRAMC is focussing on the preliminary Core Road Network (CRN) of 25,000km that consists of National Highways (NH), State Highways (SH), and Major District Road (MDR). On this CRN various kinds of road data including road inventory, condition, pavement strength, pavement rut, pavement roughness and video surveys are being collected with the aid of advanced technological equipments. The equipments which are being used in road data collection are (1) ROad Measurement Data Acquisition System (ROMDAS) (2) Falling Weight Deflectometer (FWD) (3) Ground Penetrating Radar (GPR). As the data being collected is of large volume, utmost care shall be exercised during data collection, processing, validation and compilation.

1.2 Scope of this Document

The *Road Data Collection Procedures* document provide the guidelines about the road data collection procedures, whereas this document provides the procedures of the data processing, validation, and compilation of data. This document presents a logical system which will facilitate the processes of data checking, data processing, validation and compilation. This document targets to highlight the procedures for

- Receiving data from field teams;
- Checking of raw data for its completeness;
- Data processing;
- Validating processed data;
- Compiling the data in the predefined formats for further uses.



2. Data Processing- ROMDAS

2.1 Survey Data Submission

After completion of the field survey the field data shall be submitted to the office staff at regular intervals e.g., weekly or fortnightly or whenever possible, by using portable external hard disks. At the time of submission the following data shall be submitted:

- Raw (ROMDAS) survey files (.rbf);
- Video files (.avi);
- Survey log sheet containing the details of the surveyed length;

The summary of the details received from the field staff shall be maintained by the office staff in a log file. A sample log file is given in Table 2-1.

Table 2-1: Data receiving Log file

Survey Date (dd/mm/yyyy)	Link ID	Link Description	Starting From	Ending At	Length (km)	Number of RBF Files	Number of Video Files	Survey IDs	Data handed over date (dd/mm/yyyy)	Data Submitted by (field staff name)	Data received by (office staff name)	Comments

The office staff shall review the completeness of the details provided by the field staff, and can reject the data if any of the required information is missing, and may also suggest the field team to do resurvey.

2.2 Data Receiving

2.2.1 File structure

Any data collection activity requires a systematic data filing, to ensure that the appropriate and relevant data is available to all the staff working on the project at the ease. The file structure which is followed for storing the data in the project server is as shown in Figure 2-1.

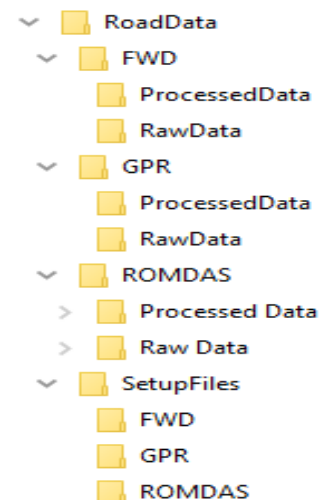




Figure 2-1: File Structure

2.2.2 File Naming Convention

The file names shall be selected in such a way that they are self-explanatory. An index document shall be created to describe the files or contents of the file.

2.3 Processing of Raw Data

All survey data is recorded in a binary format file (rbf). These files can be processed using ROMDAS software to produce Microsoft access i.e., .mdb files. The file names for the processed files are taken as the name from the **Survey File** field in the Survey setup log. For example, a survey file of SH16 would results in the file SH16.mdb.

The step by step process of processing the .rbf files to .mdb files is as mentioned below.

- Copy the .rbf file in the C:\ROMDAS\Survey Data\Data folder in the computer in which ROMDAS software is installed
- Open ROMDAS software and select *Data Processing* option which prompts to select the raw .rbf file

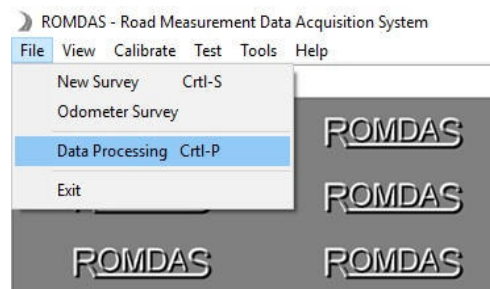


Figure 2-2: Selecting Data Processing Option- ROMDAS

- Open the .rbf file from the directory C:\ROMDAS\Survey Data\Data for processing

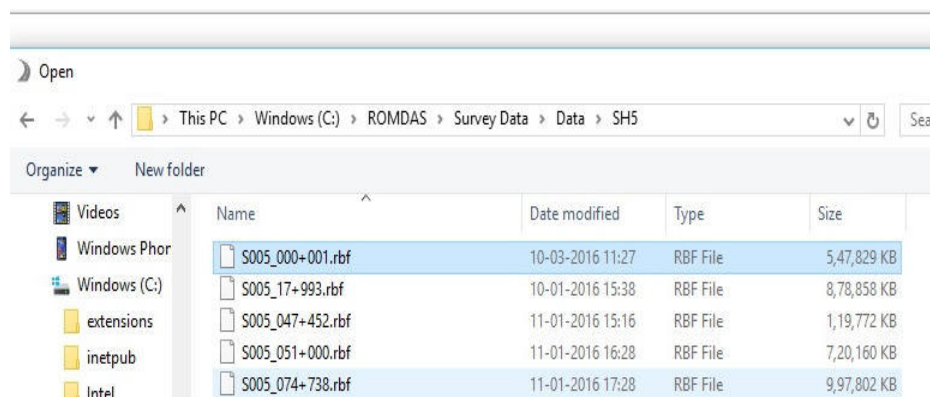


Figure 2-3: Selecting raw file for processing

- Add the .rbf files to ROMDAS software

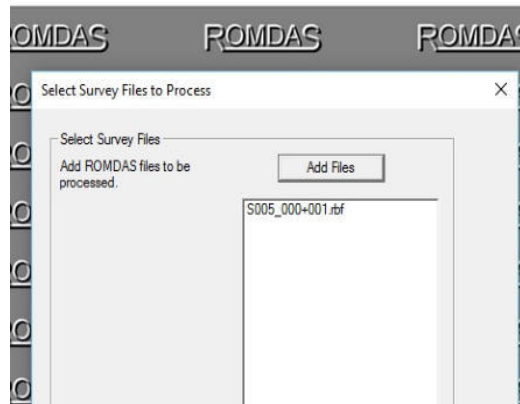


Figure 2-4: Adding files for Processing

- On the basis of the data collected, select the applicable data items for processing and click on *Process*, which will commence the processing of data and the time taken for completing the processing depends on the size of the file and number of data items included in processing

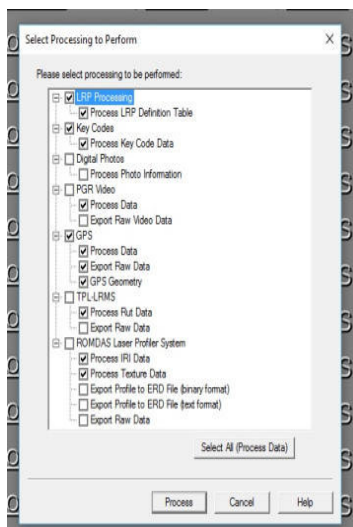


Figure 2-6: Selecting applicable data for processing

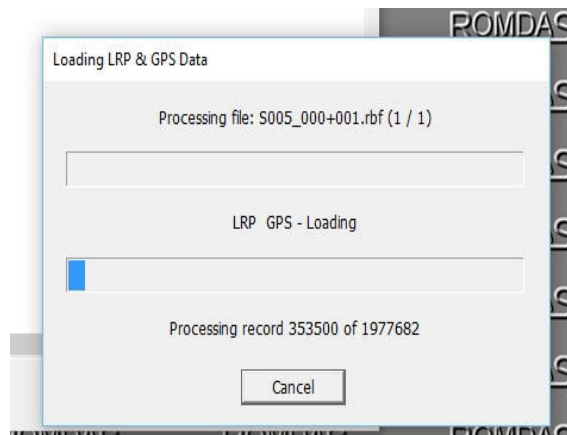


Figure 2-5: Data Processing underway

- After processing is completed, .mdb files will be created at the same file location where the .rbf file is located.

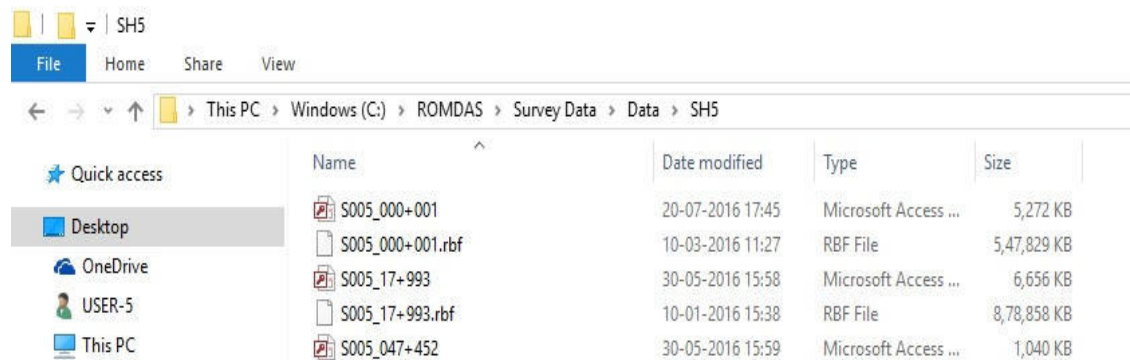


Figure 2-7: Microsoft Access (.mdb) files created

2.4 Validation of Processed Data

After processing the field data the data shall be validated. During this validation the following items will be audited and confirmed;

- Survey data is available for the entire link;
- If any missing information in Inventory and condition data;
- Random checking of the inventory and condition data with the videos;
- Acceptable video images and no gaps in video files;
- No significant gaps in GPS data;
- Anomalies in the collected data;
- Mismatches in the chainages across various data tables of the same link.

During the above validation exercise if it is found that any data is of not acceptable quality, then reprocessing of the data shall be carried out. If no acceptable results are obtained after the reprocessing then resurvey of the respective survey sections is required.

2.5 Data Compilation

After completing the data validation, the finalised data will be compiled and submitted for the necessary further uses. The processed data shall be submitted as follows:

- As per the Formats, given in Annexure A;
- Video files in AVI format;



3. Data Processing- Falling Weight Deflectometer (FWD)

3.1 General

Under PRAMC, the pavement deflection data collected by Falling weight Deflectometer (FWD) is analysed using Deflection Analysis of Pavement Structures (DAPS) back calculation software. This is a rapid, accurate and reliable method for performing back-calculation of deflection results to find out the layer stiffness modulus using a singular decomposition technique.

3.2 Theoretical Background

The DAPS algorithm solves for both a two layer elastic system and the thickness of subgrade, or a three layer elastic system and the thickness of subgrade. A least square solution process is applied, employing all the measured deflections as parameters characterizing the bowl.

A rigid base beneath the subgrade is assumed ('bedrock'). This is an accepted method, to some extent, to allow for known effects of non-linearity within the subgrade soil. The rigid base depth is used as an unknown to be solved for, along with the layer modulus.

Seed values for the AC and subgrade stiffness's are obtained from equations published by Thompson (1989), using deflections d_0 to d_3 . These are used to generate trial values of the parameters characterizing the bowl (i.e. the deflections). If a granular base is assumed to be present, the granular base resilient modulus seed value is estimated by empirical relations (Thompson, 1982). If a three-layer system without granular base is to be solved, the subgrade E estimate can be used for the base layer also. An arbitrary fixed initial trial value of subgrade thickness is employed, viz. 7m.

As described so far, it is evident that there are 7 known parameters, and either 3 or 4 unknowns, viz. 2 E's and 1T, or 3E's and 1T. Since there are more parameters than unknowns, an over-determined set of simultaneous equations can be set up relating changes in the unknowns to changes in the deflections by means of a matrix of partial derivatives, $\frac{dp_i}{dU_j}$, where p are the deflections and U are the unknowns (either E values or thickness value).

A least squares solution to these simultaneous equations is obtained by an iterative process using, at each iteration, a solution of the over-determined equation set by the Singular Value Decomposition technique (Press et al., 1986).

The difference between the computed deflections based on the initial unknown's estimates (seed values), and the measured deflections are hence minimized by the following procedure for updating the unknowns:

$$P_{kak} = r_k$$



where:

P_k = the k th iteration of the matrix of partial derivatives dp_i/dU_j of the parameters p_i , $i=1$ to 7, with respect to the 'unknown' layer modulus and thickness U_j , $j=1$ to 3 or 1 to 4.

a_k = the k th difference vector, which is the differences $U_{j,k+1} - U_{j,k}$ between the modulus/thickness used in the P_k matrix and the new modulus/thickness $U_{j,k+1}$ to be used in the $(k+1)$ th iteration.

r_k = the residual vector of differences between the most recently computed parameters and the parameters represented by the measured deflections.

In the above equations, the partial derivatives comprising the P matrix are estimated numerically, by Elastic Layer analysis. At present, no limits are applied to E values generated by the minimization procedure.

This back-calculation procedure is considered to be suitable for:

- Full depth asphalt concrete (AC)
- Conventional (AC surface plus granular base)
- AC + high-strength stability base (HSSB)
- PCC over granular base

3.3 Validation of the Acquired Data

After collecting the data from the field, prior to carrying out further processing the following validation checks shall be performed

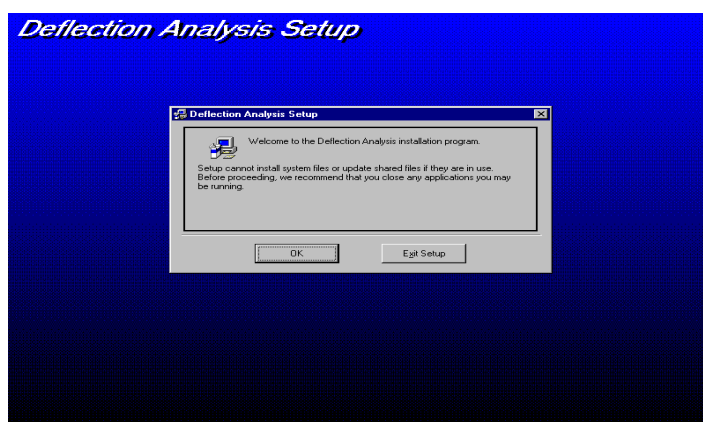
- The values of deflections should be in descending order from geophone D1 to D9;
- If any of the deflection from the sensor D1 to D9 is missing / showing more value than its preceding geophone value, such values should be corrected with the average reading of the test data from the remaining Drop tests conducted at the same location;
- The first row of Deflections should not be blank. In case of any missing in data the complete drop test is omitted or the immediate drop in the same location is to be used for the back calculation;
- If the First Geophone Data/ majority of the Geophones data collected is invalid/Missing for all the drops at a Location, such data should not be processed further.



3.4 Installation of DAPS Software

The DAPS program is supplied on a single CD-ROM. The user runs the setup.exe program using normal windows execution (e.g. from run menu a:\setup.exe) and then follows the instructions given by the software.

The setup creates a default directory of c:\daps and c:\daps\data1. The data directory is the default data directory for storing deflection data for analysis. The user can change this directory if necessary along with the program directory during the installation process. Following the installation it is recommended that the user restarts the computer to ensure that all setting changes have taken place.

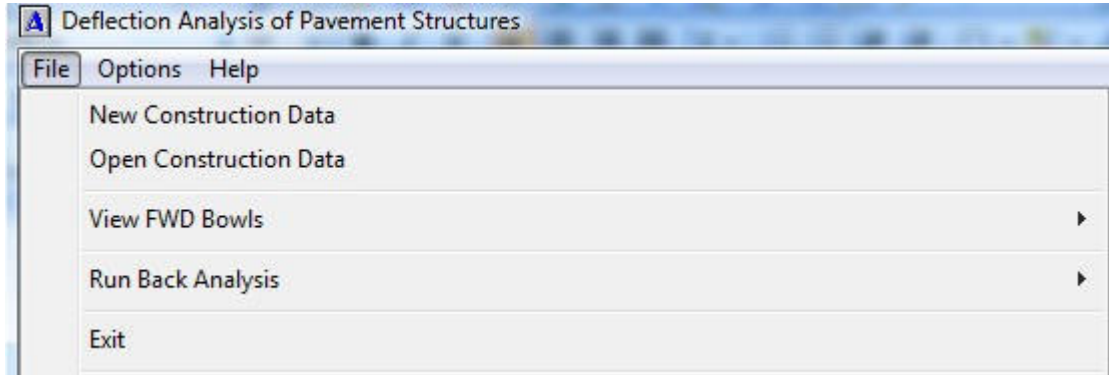


3.5 Program Use

The user runs the program from the Start-Program menu or using other standard window methods and then starts by selecting the file menu as illustrated. The file menu enables the user to choose an area to work in. The choices are illustrated and are as follows:

- New Construction Data
- Open Construction Data
- View FWD Bowls
- Run Back Analysis
- Exit

Each of these areas will be considered in detail in the following pages.



3.6 Construction Data

Pavement construction information must be defined to enable back calculation. This is done via the creation of a construction information file. The file should have the same root name as the FWD data file and would be normally located in the default directory c:\daps\data unless otherwise specified by the user (Figure 3-1).

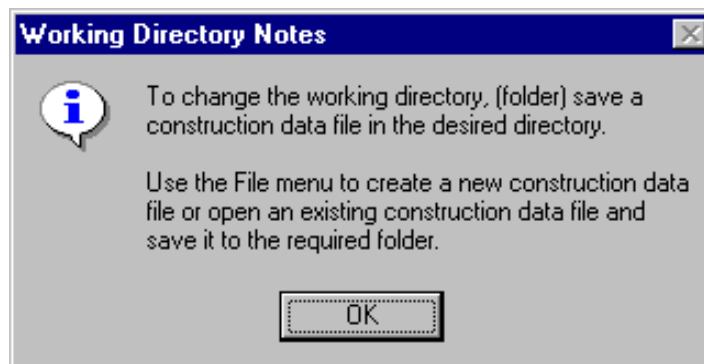


Figure 3-1: Working Directory Notes- DAPS



Deflection Analysis of Pavement Structures

Help Window

Construction Type_1

Number of Pavement Layers (2 to 3 allowed)

Layer No.	Material Type	Thickness m	Poisson ratio	Density kN/mcu
1	AC	0.1	0.35	24
2	SB	0.4	0.4	19
	SG	999	0.49	18

Material Type: Any 2-letter alpha code eg AC PC SB GB SG

No. of Sub Sections with this structure (8 max)

Sub Section Number	Start	End
1	0	100000

A station is within a Sub Section if
Start <= Distance < End

Figure 3-2: Example of Pavement Construction Types

The pavement construction information can be defined for various lengths of the pavement by reference to the location numbers. In the example given (Figure 3-2), three construction types have been defined for the FWD data file. The user must define a length of pavement associated with each construction type. The information stored will be as follows:

Material Types:

- AC – Asphalt Concrete
- PC – Portland Cement
- GB – Granular Base
- SB – Granular Sub-base
- SG – Subgrade

Poisson's ratio must be less than 0.5. Thickness and density inputs are in metric units. When the user has completed the data entry in this area he uses the **Save** or **Save As** file commands to store the file. An alternative to starting a new construction information file is to use the data in an existing file by opening and then use the **Save As** command to a new file name after completion/modification of data.

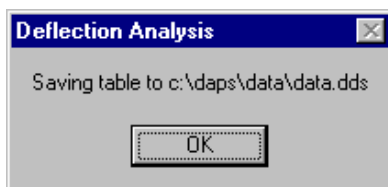


Figure 3-3: Saving Construction Data

3.7 View FWD Bowls

When selecting the menu option *View FWD Bowls* the user is presented with the form as illustrated in Figure 3-4. Data files in the working directory are listed under the data item in the grid. The program recognizes most common FWD file formats, including one EXCEL spreadsheet format. Initially, all other information items are blank. Hitting the Summarize button fills in this information. This summary can be saved to the hard disk. The data is stored in the file *data.dds* and is illustrated below.

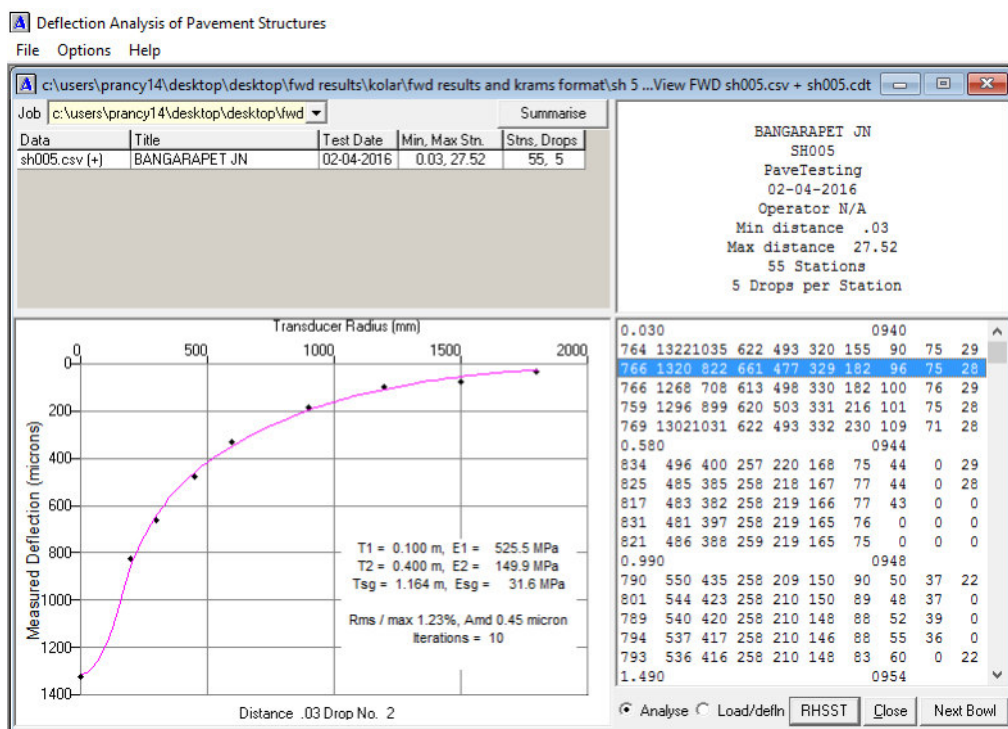


Figure 3-4: Option View FWD Bowls

This process determines the number of deflection bowls in the data file and allows the user to review the distance/station information to be reviewed with ease.

The plus symbol next to the data filename indicates that a CDT (construction data) file has been completed (Figure 3-5) which is necessary to perform the back-calculation. A box towards



the upper right corner of the screen contains general information in the file. Title, test date, distance information, number of stations and drops per station are displayed in the grid.

Deflection Analysis of Pavement Structures

File Options Help

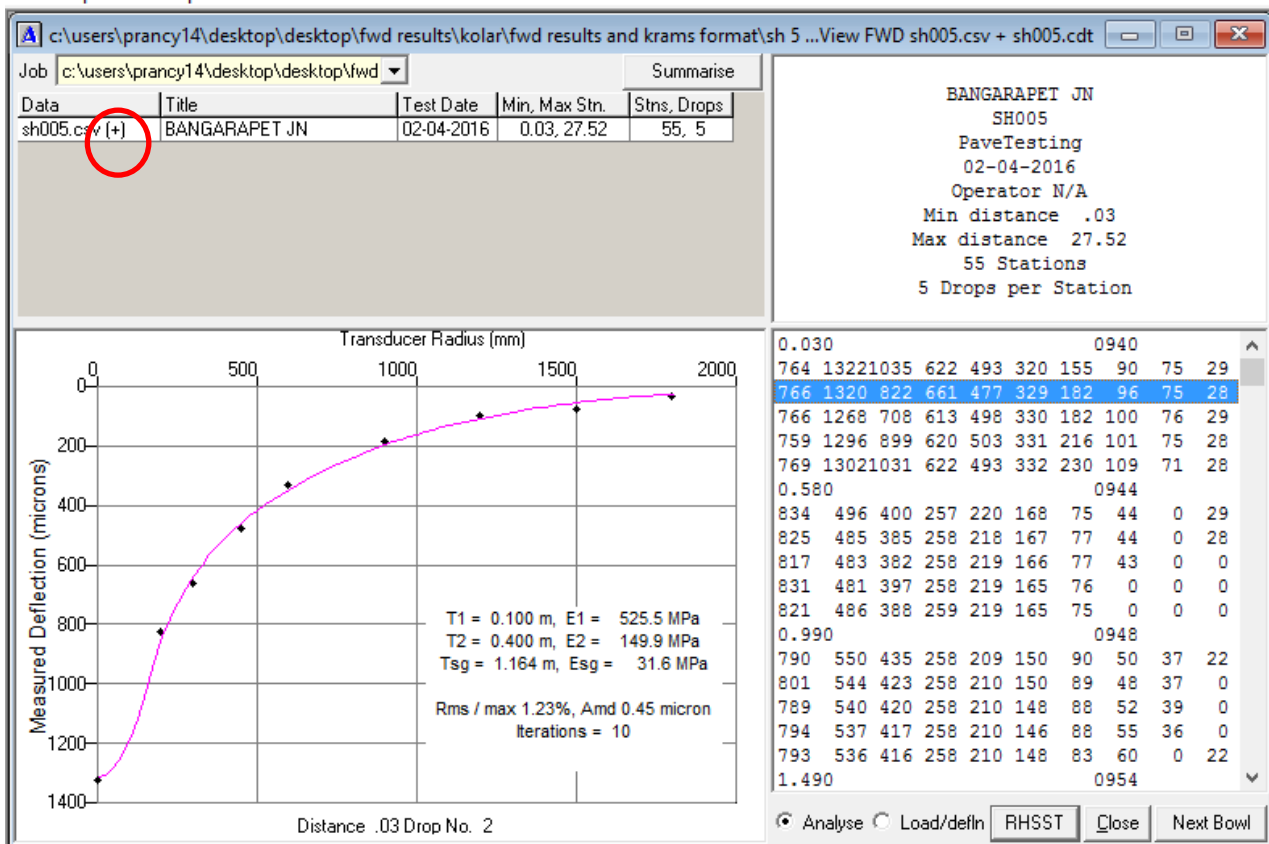


Figure 3-5: Indication of Construction File completion

Single data points can be analysed by interactively clicking on a deflection bowl (providing a CDT file has been made). Information displayed on the deflection bowl graph includes the layer stiffness and the calculated stiffness for each pavement layer. The thickness of the soil layer is the calculated thickness to an assumed rock foundation. The statistic RMSerr/Max

(Root Mean Square error divided by Maximum Deflection) is also given. A RMSerr/Max of less than 4% is considered as a satisfactory deflection bowl match. It is recommended that users use this feature to inspect the data files and determine which model most accurately fits the back-analysis.

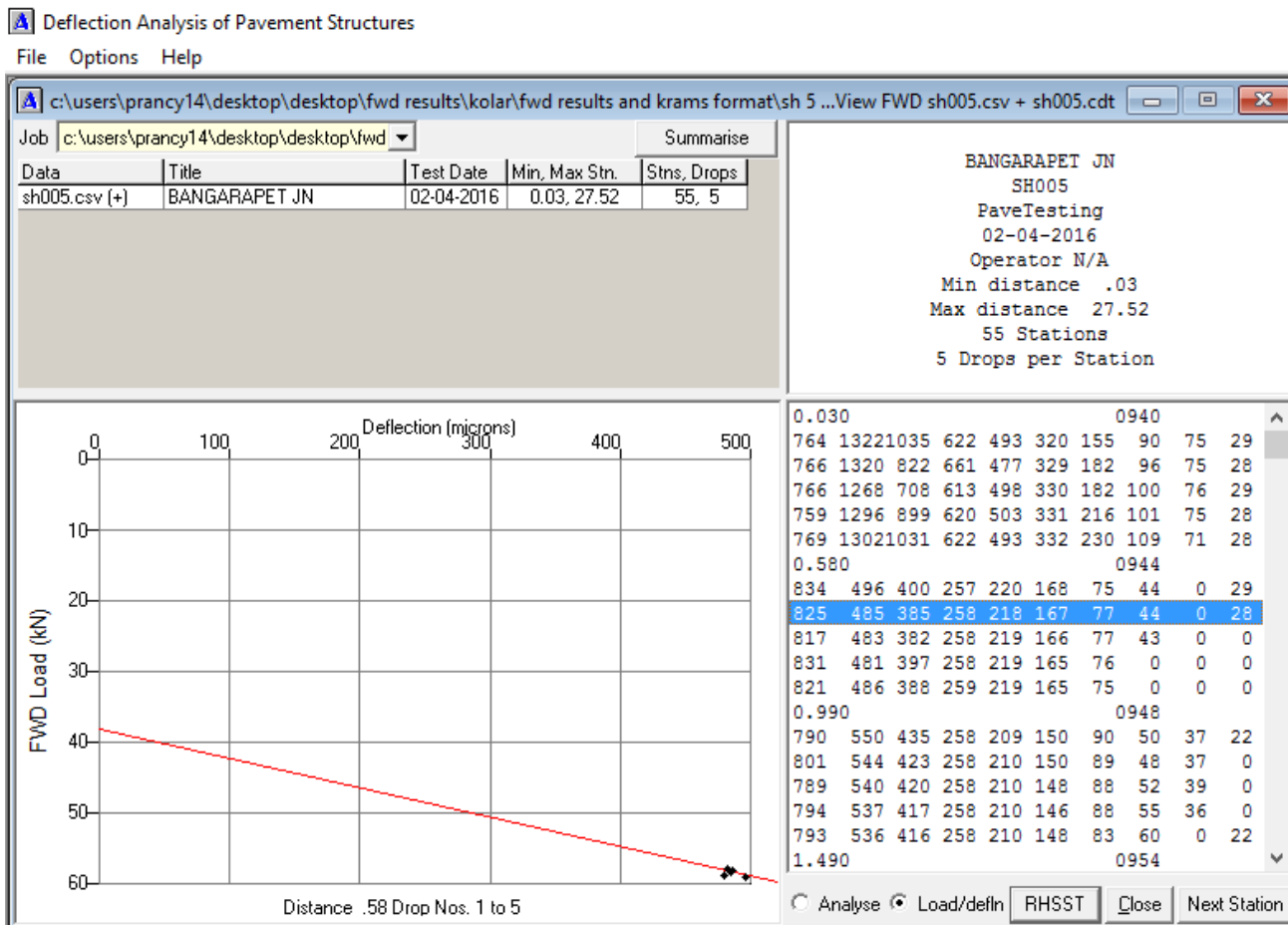


Figure 3-6: Indication Graph Showing Load/Deflection

3.8 Processing of FWD Data

A file which can be processed is indicated by a (+) symbol next to the data file name. The file is selected and processed by selecting the “run” button. Information is updated as the analysis takes place in the display grid. The modulus of the layers are displayed (E1 to E3) along with the thickness of the soil layer to bedrock (Sthk), the calculated horizontal strain (tensile) at the underside of the bound layer (epsH) and the vertical strain at the top of the soil layer (epsV). In addition the %RMS (Root Mean Square error divided by Maximum Deflection) is given in the last column. As discussed earlier a %RMS error of less than 4% is considered as a satisfactory deflection bowl match. The summary window (middle right) indicates general statistical information on the analysis such as number of bowls successfully analysed. The user can save the results by clicking on the *Save Results* button. In addition, the user is able to review the data and make any changes to the construction information, if required, and rerun the analysis. The file, when saved, is stored in a *BLS* file that is formatted to enable easy import into a spreadsheet

for further manipulation and/or analysis. The order of the data in the .BLS file matches the columns given in the Figure 3-7 are shown in Table 3-1.

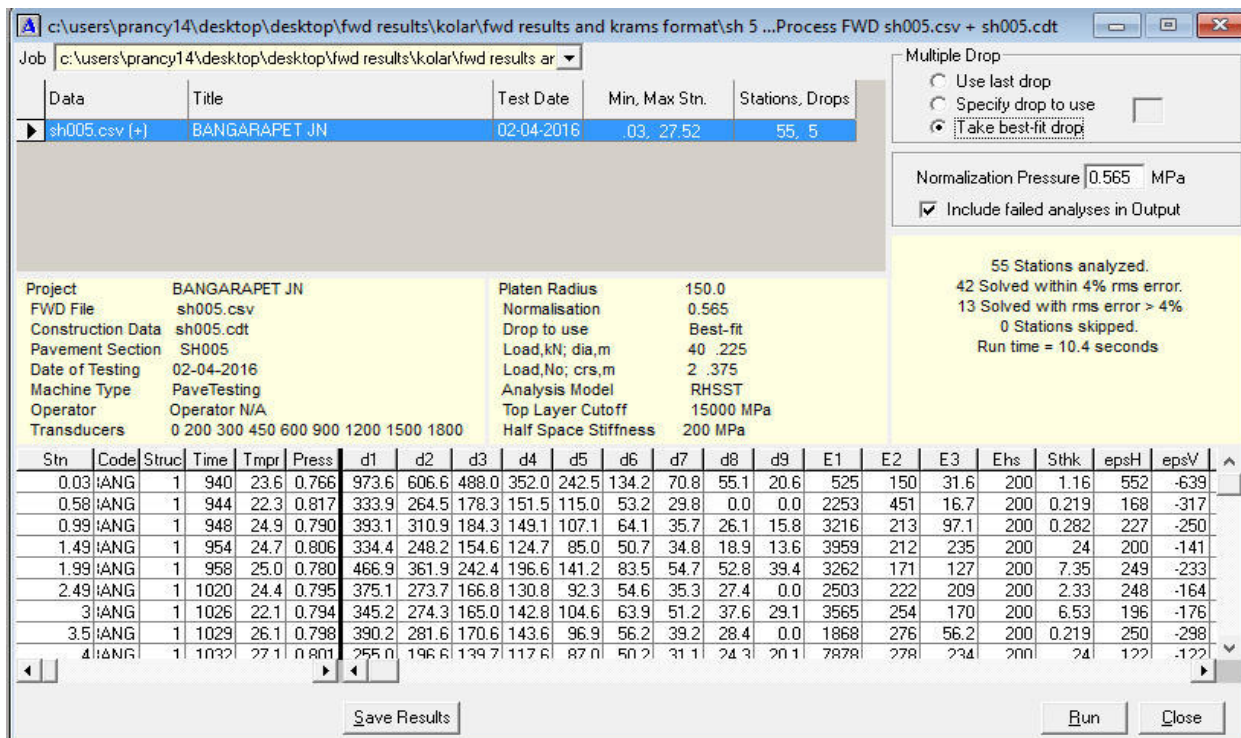


Figure 3-7: Back Calculation Results

Table 3-1: .BLS file columns Description

Column #	Description
Stn	Station number
Code	If entered a code is shown
Struc	A code number for a structure
Time	If entered a time will be displayed
A.Tmpr	Air temperature
S.Tmpr	Surface Temperature
Press	Pressure on loading platen
d0	Deflection of 1 st sensor
d1	Deflection of 2 nd sensor
d2	Deflection of 3 rd sensor
d3	Deflection of 4 th sensor
d4	Deflection of 5 th sensor
d5	Deflection of 6 th sensor
d6	Deflection of 7 th sensor
d7	Deflection of 8 th sensor
d8	Deflection of 9 th sensor



Column #	Description
E1	Stiffness modulus of top layer
E2	Stiffness modulus of second layer
E3	Stiffness modulus of third layer
Ehs	Stiffness modulus of half space
Stkh	Thickness calculated to bed-rock
epsH	Calculated horizontal strain at underside of lowest asphalt layer
epsV	Calculated vertical strain on top of half space (assumed soil)
%rms	Root mean square error of bowl fit compared to measured deflections
Amd μ	Absolute micron deviation of calculated bowl from fitted bowl
c1	Calculated 1 st deflection using solved values
c2	Calculated 2 nd deflection using solved values
c3	Calculated 3 rd deflection using solved values
c4	Calculated 4 th deflection using solved values
c5	Calculated 5 th deflection using solved values
c6	Calculated 6 th deflection using solved values
c7	Calculated 7 th deflection using solved values
c8	Calculated 8 th deflection using solved values
c9	Calculated 9 th deflection using solved values

The other options on this window allow the user to use the last drop for the analysis, specify which drop to use or to use the best fit drop. If the best fit drop is selected then the software will attempt an analysis with each deflection bowl at a given location but only save the data with the lowest % RMS error. The user may also specify a normalization pressure. This adjusts all the output to a constant applied stress rather than the deviating stress that is typically found in FWD deflection data. In addition, failed output can also be included. This is a useful feature since it enables an engineer to see which deflection bowls could not be analysed. The engineer may choose other methods to estimate a stiffness modulus for these locations. Results Obtained in .b/s format are as Shown in the Table 3-2.

Table 3-2: Results obtained in “b/s” format

[illegible]



3.8.1 Options

Several user options are available as discussed below:

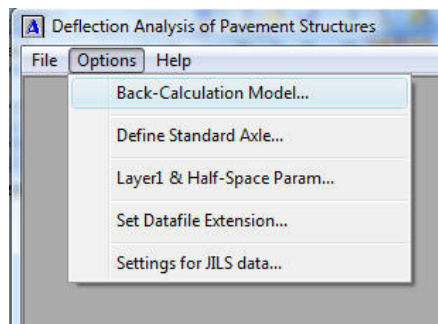


Figure 3-8: Options

Back-Calculation Model: DAPS supports three types of Back Calculations models (1) Rigid H-S and Sg Thickness (2) Known-Half Space (3) A,B,Subgrade . Based on the road conditions in the state of Karnataka it is recommended to use the *Rigid half space and Sg Thickness model* for the analysis of the FWD data.

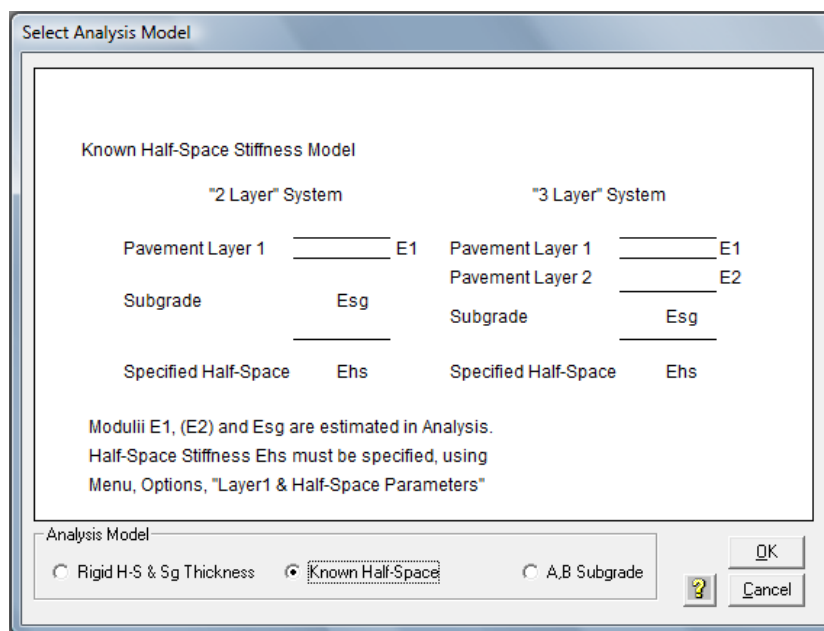


Figure 3-9: Selecting Back-Calculation Model

Define Standard Axle: The standard axle defined is used to determine these strains at critical locations. The standard Axle load define for Karnataka condition is shown in Figure 3-.



The 'Define Standard Axle' dialog box contains a diagram of a standard axle with two circular tyre contact areas. The left circle has a '+' symbol in the center, labeled 'Principal strain location'. The distance between the centers of the two circles is labeled 'spacing'. The diameter of one circle is labeled 'diameter'. Below the diagram, there are input fields for 'Total load on two wheels' (40 kN), 'Tyre contact diameter' (0.225 m), and 'Spacing centre to centre' (0.375 m). To the right, there are radio buttons for 'Single wheel' and 'Dual wheel' (which is selected). At the bottom right are 'OK' and 'Cancel' buttons. A note at the bottom states: 'Horizontal Strain at underside of top Pavement Layer, and Vertical Strain at top of Subgrade are computed for the Standard Axle loading. Strain is output in microstrain units'.

Figure 3-10: Defining Standard Axle

In the out-put file (*.bls) a calculation is provided of the strain in critical locations on the basis of the defined standard axle load.

Layer 1 & Half Space Parameters: The upper layer modulus can have a cut-off value selected. This is useful if analysing very thin pavement layers. In addition, it is possible to set a half-space stiffness (associated with the rock at depth).select the option **Specify a Limit**

The 'Layer 1 & Half-Space Parameters' dialog box has two main sections. The 'Top Layer Cutoff' section has two radio buttons: 'Specify a limit' (selected) and 'No Limit'. Below 'Specify a limit' is a text box for 'Maximum stiffness' with the value '15000' and the unit 'MPa'. The 'Half-Space' section has a text box for 'Stiffness' with the value '100' and the unit 'MPa'. On the right side, there is a yellow question mark icon. At the bottom right are 'Ok' and 'Cancel' buttons.

Figure 3-8: Layer 1 & Half Space Parameters

and provide maximum Stiffness limit to 15000 Mpa and Half space Stiffness to 200 Mpa



Set Data file Extension: The user can define the data file extension. This is useful in that non-needed data files can be filtered out by careful selection of data file extensions as csv

Set filename extension for FWD data files

Current Extension is

Give 3-character extension (wildcard character ? is allowed)

New Extension

NB. The characters below are not permitted:
< > : " / \ | .

OK Cancel

Figure 3-9: Layer 1 & Half Space Parameters

Settings for JILS data: In the JILS data file format various parameters (transducer spacing and load-plate diameter) are not defined. These must be defined in this dialog box to enable back-calculation.

Additional Information for JILS data

Transducer Settings

d1-d9	Radius, mm	Include
d1	0	☒
d2	200	☒
d3	300	☒
d4	450	☒
d5	600	☒
d6	900	☒
d7	1200	☒
d8	1500	☒
d9	1800	☒

Name:

Platen Radius: mm

New OK Cancel

NB. For 7 transducers, 2 dummy items are appended.

JILS DATA TYPE OPTION

☒ Default - Data with M2 as 1st record is considered Metric, data with M3, M5 or M7 is assumed US Customary.

☐ Customary override - Always assume US Customary units

☐ Metric override - Always assume Metric units

Where Metric units are kN force, microns deflection.
US Customary units are kips force, mils deflection.

Figure 3-10: Transducer spacing and load-plate diameter



4. Data Processing -Ground Penetrating Radar (GPR)

4.1 General

After acquiring the radar data (scan) using GPR equipment, it has to be processed for determining the pavement layers thicknesses. For this purpose the GPR equipment is supplied along with a Georadar Data Elaboration High Density (GRED HD) software. This software is dedicated to the visualization of radar data acquired with the GPR system. In particular, it allows to process, elaborate and visualize the radar data in 2 and 3 dimensions, using the full potential of the 3D processing algorithms. The ensuing paragraphs elaborates the step by step procedure that covers an average GPR analysis from the elaboration of the raw data to the visualization of the results.

4.2 System requirements

For a proper use of the GRED HD Software the following are the minimum computer specification requirements

- Intel® Core™ i7 CPU M 620 @ 2.67GHz;
- 8 GB SDRAM;
- System type: 32-bit or 64-bit Operating System (with Administrator rights);
- 1 USB port for the installation of the HW key;
- Hard Disk SATA at least 7200 rpm and 250 GB (keep always 20 GB of free space);
- Graphic Adapter with 512 MB of dedicated memory;
- Ethernet 10/100 Base-T adaptor;
- 17 VGA Msync MPR » APM minimum Colour Monitor.

4.3 Software Module Sketch

GRED HD software is composed by the following 4 modules:

- BASIC,
- 3D,
- CAD,
- BRIDGE.

The **BASIC** module includes:

- Geographic Navigator,
- Import GeoTiff images,
- B-Scan data analyser
 - Processing and macro,
 - Generic target (single plot),



- Layers/cores in a 2D mode,
- Export layer to Excel,
- Export data to ASCII,
- Saving images,

Under PRAMC for the identification of the pavement layers the GRED HD **basic** module is used.

4.4 File menu

The *File* menu located in the software home screen consists of the commands *Open Project*, *Merge Project*, *Open File*, *Load Settings*, *Save Settings*, *Import*, and *Export*.

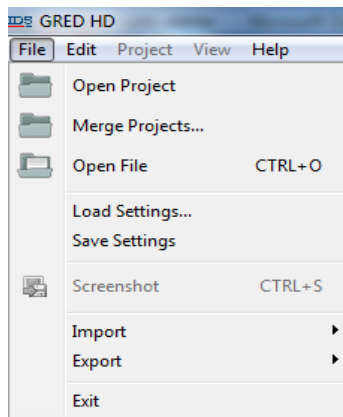


Figure 4-1: File Menu- GRED HD software

Usages of these sub-menus are given below

4.4.1 Open Project

Open Project command is equivalent to the *Open project* button  and it opens the *Project selection* window from where the project folder containing the raw data can be selected (Figure 4-2).

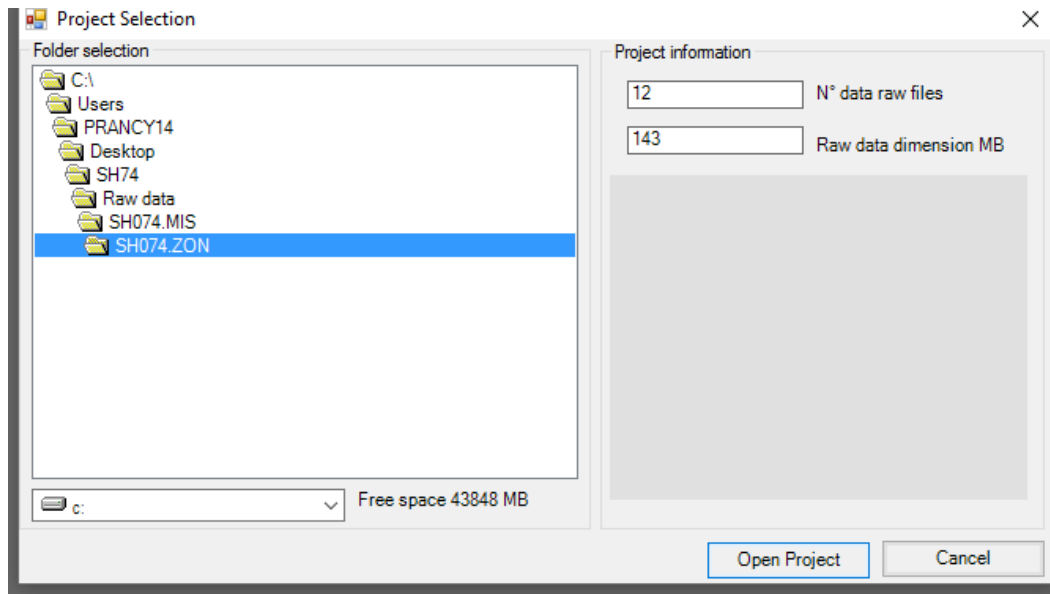


Figure 4-2: Project Selection window

Select the file for the identification of the layers, for Example SH074.ZON (Figure 4-2) and click on **Open Project**. As shown in the Figure 4-3 select the load mode as load default selection and click on **OK**.

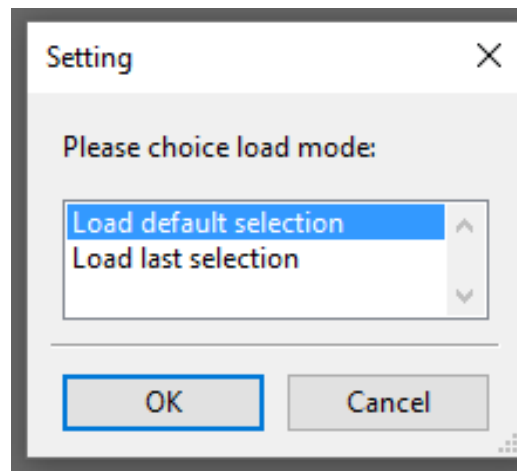


Figure 4-3: Load Mode Selection window

4.4.2 Data loading and positioning

The first time when raw data is loaded, GRED HD software automatically starts the Geographical reference module (Figure 4-4). With this utility the software can properly calculate the relative position of each scan of GPR, using the information found in the files associated with the GPS positioning system used.

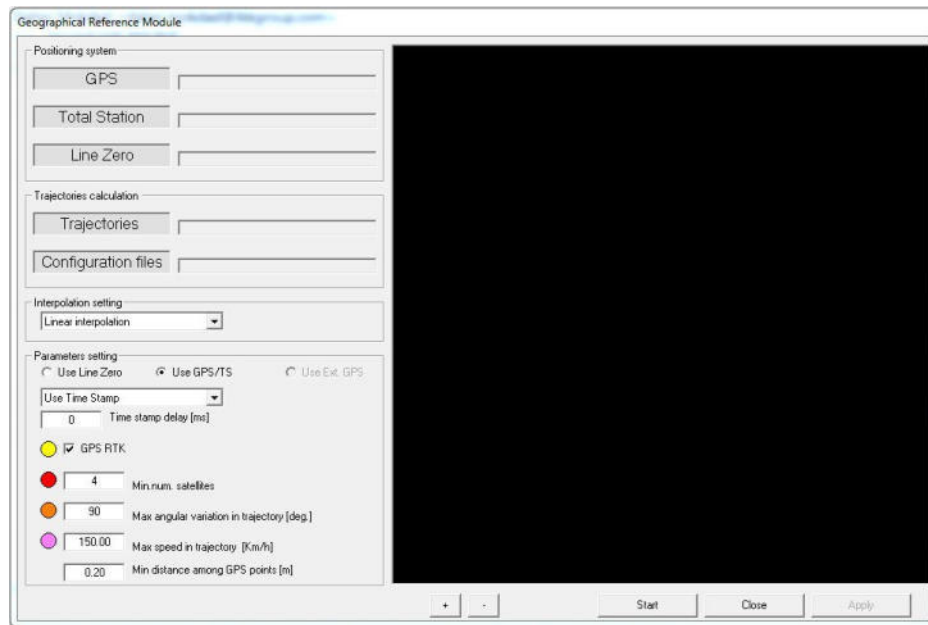


Figure 4-4: Geographical reference module

The position data is associated with the radar data with the following approaches:

Timestamp: The position data is associated with the radar data in accordance with a temporal proximity criterion. The GPR timestamp can be UTC in which case the association is completely reliable; or the timestamp of GPR is the local clock of the DAD, in which case it proceeds making an association between the two time scales associating the first radar track on the first position data received. In the latter case there may be problems due to the lag in the registration of strings.

Marker GPS: the position data is associated to the radar track that shows a mark created by the acquisition software. Bond of correspondence between the number of strings and the number of recorded radar tracks marked.

Trajectories Extrapolation: if there are gaps between the encoder wheel measurement and GPS measurement, then trajectories extrapolation technique can be useful. Note that, if the final points of the GPS are not displayed, the software will draw a straight line (in this case, this function should be used only if the final path is effectively straight).

Linear interpolation: it calculates how the points join up on straight paths

Spline interpolation: is the most "smoothing" (continuity of the first and second derivative in the junction points) and follows better the curves but sometimes it can be oscillating in due to abrupt transitions in joining the points.

Mixed interpolation: it follows "faster" the points and does not generate oscillations, it guarantees the continuity of the first derivative alone and it is less smooth. Essentially, it



applies spline interpolation to the intermediate points and linear interpolation at the beginning and end of the trajectory.

If there are a number of radar tracks acquired prior to the first measurement point position and after the last point, the software module performs an extrapolation placing these radar tracks on the path tangent to the at the first / last point measure.

When the process is over (clicking START + APPLY in Figure 4-4) the software will plot the referenced scans in the main interface as shown Figure 4-5.

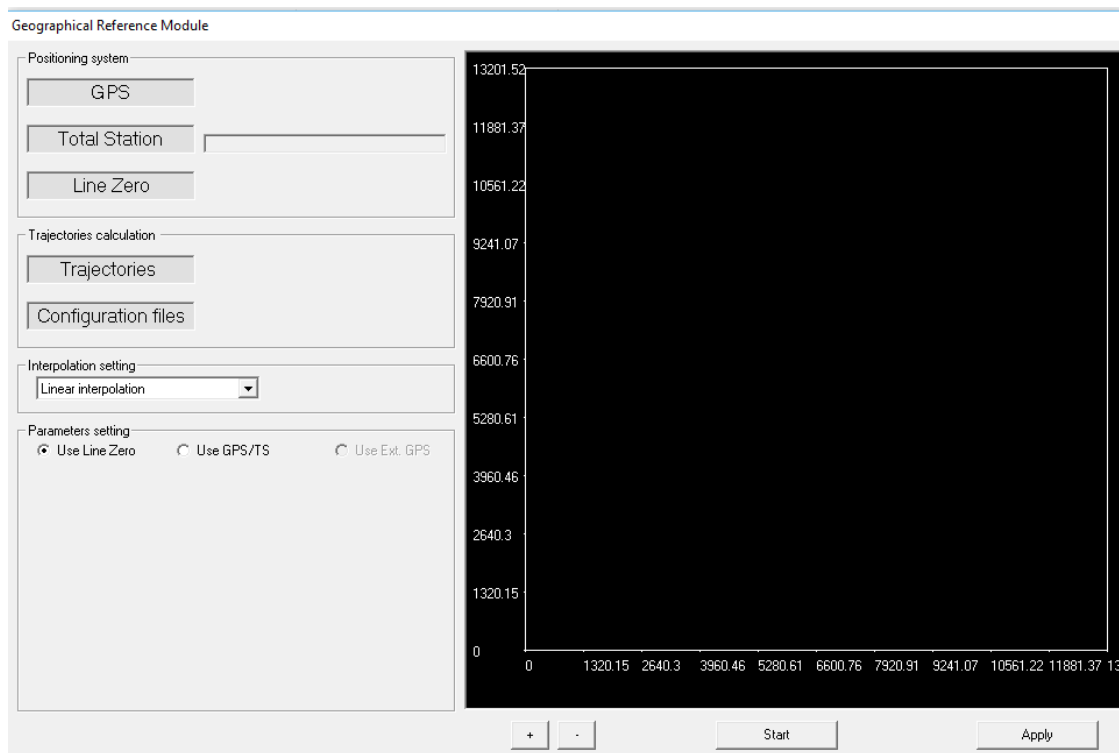


Figure 4-5: Processing of Geographical Reference Module without GPS coordinates

- The Navigation window will be displayed as shown in Figure 4-6

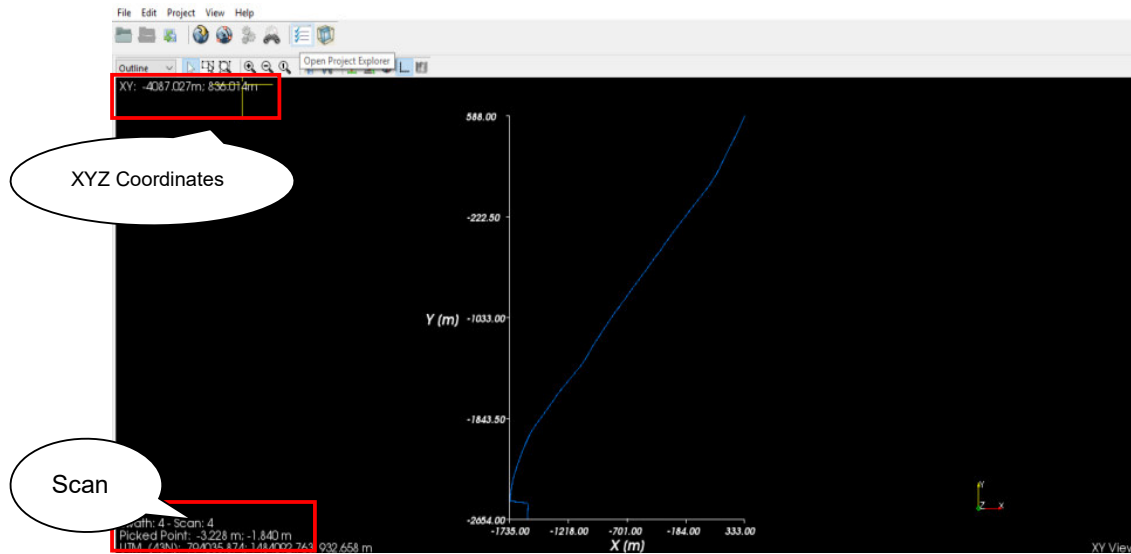


Figure 4-6: Navigation window with reference scans

- This menu gives all the options related to the visualization of the maps. The Section's commands are:
 - Project Explorer (): it opens the Project Explorer window (Figure 4-7) (shortcut ALT + F4).
 - In order to visualize swaths, in Project Explorer(Figure 4-7)selects scans, Antenna Array (when present), Data extension (.DT, .D00, .D01, .D##)
 - A single scan can be selected by clicking one time with the left mouse button on the swath (L scans) and then select View. In order to visualize only selected radar maps click on View button.

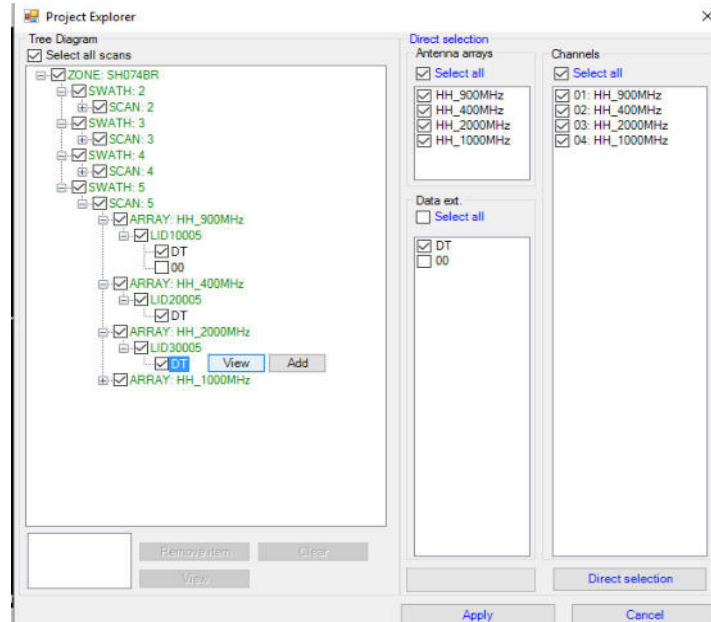


Figure 4-7: Project Explorer Tree

- The scanned layers of the road from the scan selected will be displayed as shown in Figure 4-8.

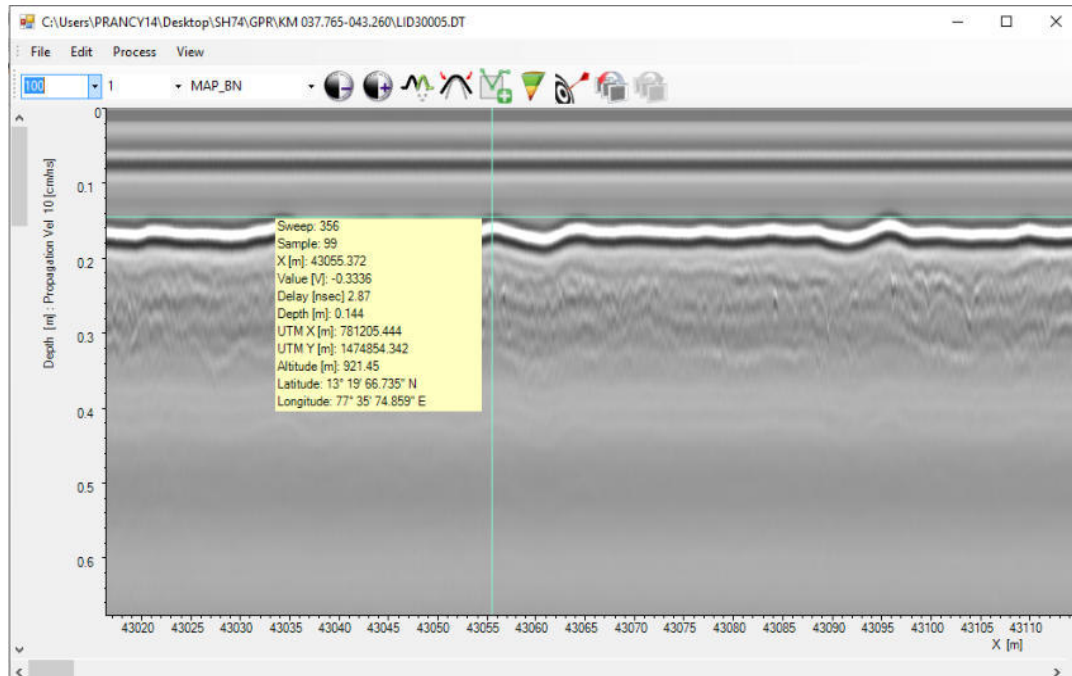


Figure 4-8: Window Showing the Scanned Layers

Following steps are to be applied for optimizing the viewer window for the identification of the pavement layers



- The layer that is being observed in the Figure 4-8 the peak point on the entire scan is to be identified by visual observation.
- By right clicking on the visualized peak the Delay (n sec) is to be recorded for the future reference.
- Open the process menu and Select the “Settings/ (Ctrl+Y)” as shown in Figure 4-9 and Figure 4-10.

4.4.3 Settings- Processing the Layers

The following is the list of the Settings to be adopted for processing the Layer Scan details collected from the field. Refer process as shown in the Figure 4-9

A. 2000Mhz Antenna:

- Move Start Time: remove the Automatic Selection and input the “n sec”(Delay in Start of Scan) i.e., n sec value is obtained by right clicking on the Peak Point Identified on the Surface Layer as shown in Figure 4-8
- Time Domain Band Pass Filter: 1000 (Low Frequency), 3000 (High Frequency) as shown in Figure 4-10
- Now Click on **Apply**, and followed by functions F1, F3, F4, F2, to apply all the necessary settings that are to be modified

B. 1000Mhz Antenna:

- Move Start Time: remove the Automatic Selection and input the “n sec”(Delay in Start of Scan) i.e., n sec value is obtained by right clicking on the Peak Point Identified on the Surface Layer as shown in Figure 4-8
- Time Domain Band Pass Filter: 500 (Low Frequency), 2000 (High Frequency) as shown in Figure 4-10
- Now Click on **Apply**, and followed by functions F1, F3, F4, F2, to apply all the necessary settings that are to be modified

C. 900Mhz Antenna:

- Move Start Time: Select the Automatic Mode
- Time Domain Band Pass Filter: 300 (Low Frequency), 1500 (High Frequency) as shown in Figure 4-10
- Gain: 200
- Now Click on **Apply**, and followed by functions F1, F3, F4, F2, to apply all the necessary settings that are to be modified

D. 400Mhz Antenna:

- Move Start Time: Select the Automatic Mode
- Time Domain Band Pass Filter: 100 (Low Frequency), 1000 (High Frequency) as shown in Figure 4-10



- Gain: 200
- Now Click on **Apply**, and followed by functions F1, F3, F4, F2 to apply all the necessary settings that are to be modified
- Applying the Modified Settings based on the antenna Selected, the processed Data will appear as shown in Figure 4-11.

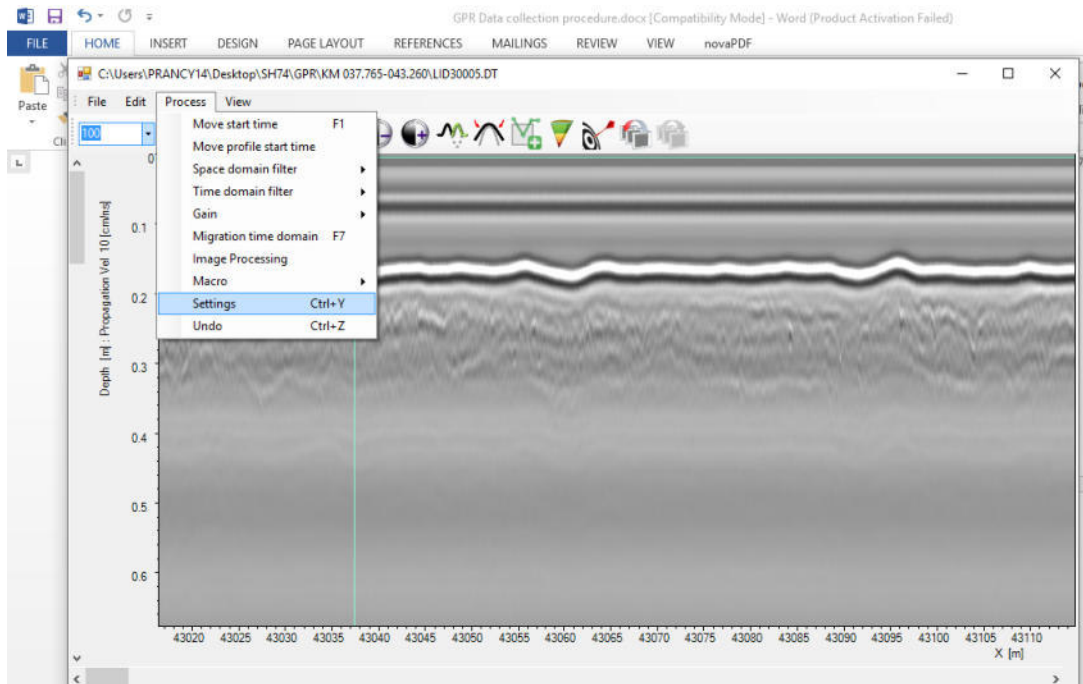


Figure 4-9: Process Setting Menu

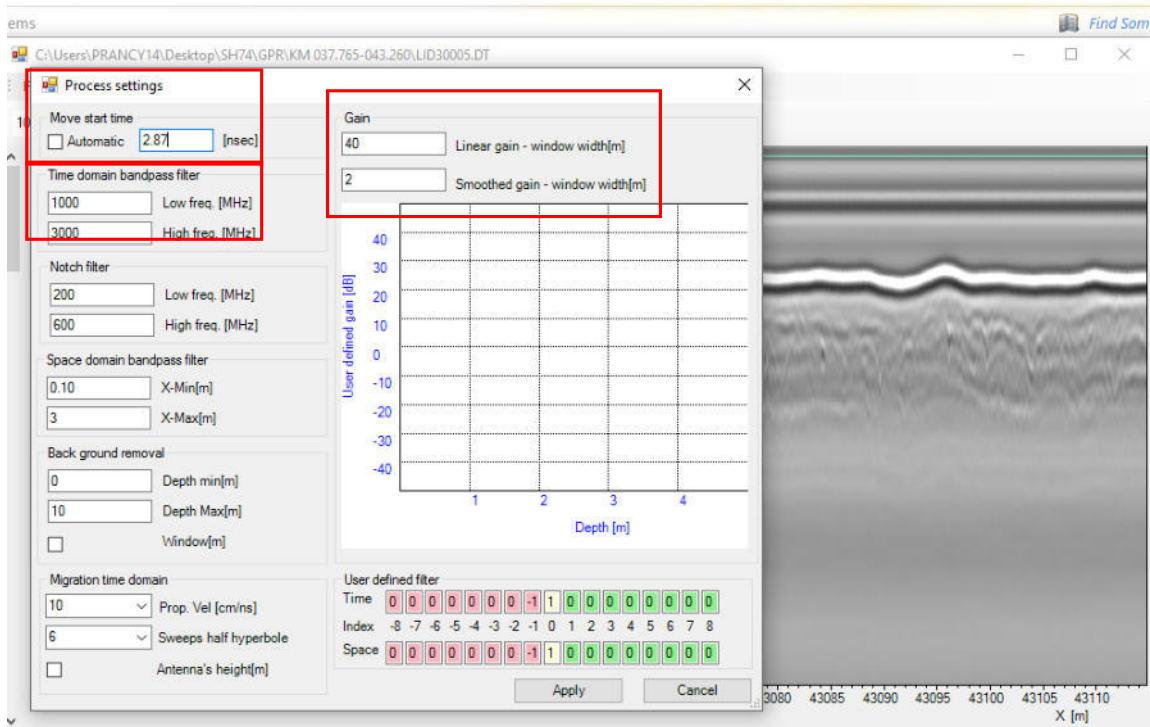


Figure 4-10: Process Setting Window

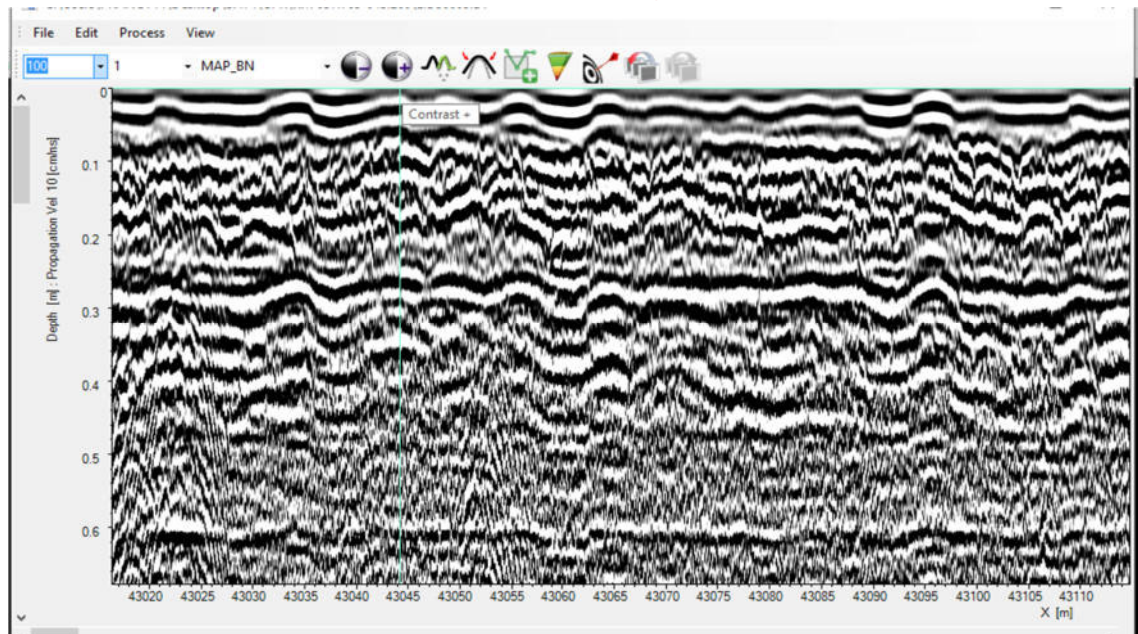


Figure 4-11: Processed Layers Window after the Application of the Required Setting

- Layers to be plotted as overlay on the identified GPR radar data, using the “New Layer” Tool as shown in Figure 4-12.



- Layer can be plotted in both *Manual* and *automatic* modes, in which layer start point is to be identified manually and further it can be shifted to automatic mode or can be continued with manual plotting of the pavement layer;
- After completion of plotting the layer save the layer by right click.

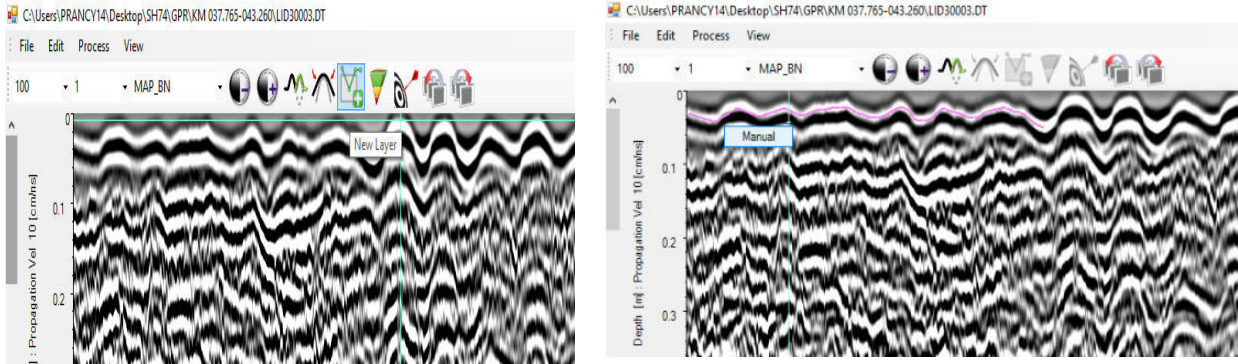


Figure 4-12: Plotting the Pavement Layers

- Plot the required layers i.e., Top Surface of the Asphaltic Layer, Top Surface of the Granular Sub base, Top Surface of the Sub Grade Layer.
- The plotted layers require calibration at intervals of 5Kms

4.4.4 Calibration of the Plotted Layers

- Use core details at the chainage where the test Pit Data is collected by selecting the Option “**Insert Core**” input the Layer depth of each layer from top (refer Figure 4-13).
 - For Example: from the Collected Test Pit Data thickness of the Asphalt Layer is 100mm, Granular Sub base is 250mm
 - For Layer 1 starts from the top of the scan enter the Dummy Value as 0.05m
 - For Layer 2 of Asphalt Layer Thickness will be $0.05 + 0.10 = 0.15$ in meters
 - For Layer 3 of Granular Sub base thickness will be $0.05 + 0.10 + 0.25 = 0.4$ in Meters
- Now click on *Apply* to apply the Calibration and the layer Legend of the calibrated layers can be seen using the “*View*” option followed by “*Layer Legend*” in the menu bar as shown in Figure 4-14.

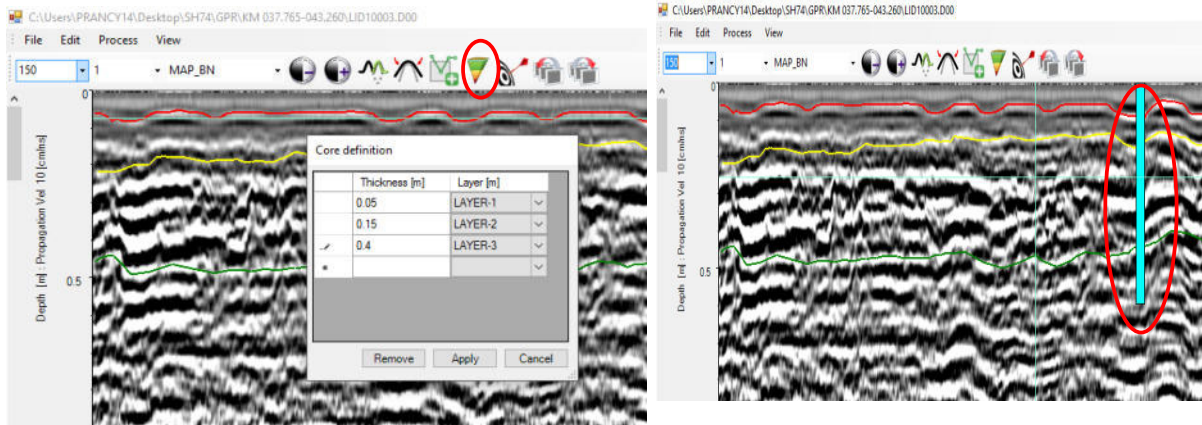


Figure 4-13: Inserting the core Details

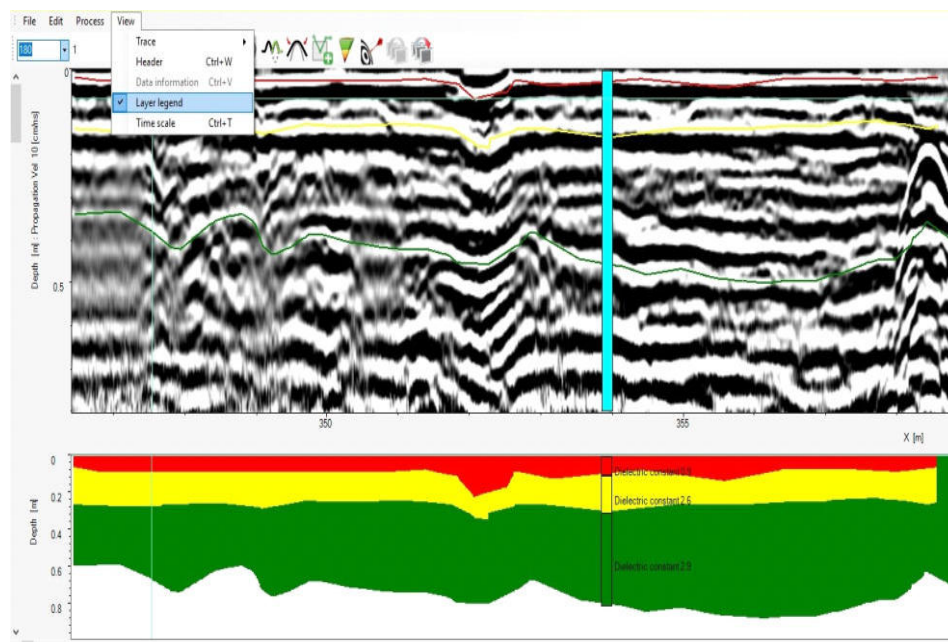


Figure 4-14: Layer Legend

4.4.5 Results Generation

1. All the above mentioned Process of Plotting Layers and Calibration is performed in the Data extension (.DT) and now it will be further saved as data extension .D00 by using “menu bar $\Rightarrow$ File $\Rightarrow$ Save” options, and similar exercises can be performed any number of times and can be saved as data extension (.D01, .D02, .D03.....) as shown in Figure 4-15.

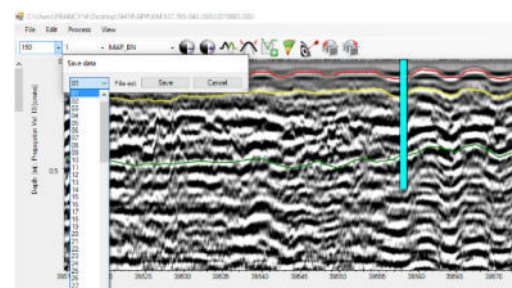


Figure 4-15: Saving the Plotted Layers



- The plotted layers can be exported to the MS Excel Format (.csv) and prescribed interval can be selected as shown in Figure 4-16.
- The exported layers details obtained with the selected interval will show the depth of each layer from the top surface of the road Scan as shown in Figure 4-17.
- The actual asphalt Layer Thickness is available by subtracting the Layer 1 from Layer 2 of the exported results spread sheet and similarly the Granular Sub base thickness is obtained by subtracting the Layer 2 from Layer 3 as shown in Figure 4-18.

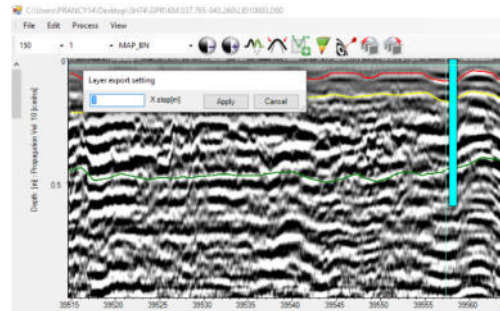


Figure 4-16: Exporting the Layers to Ms Excel

FILE HOME INSERT PAGE LAYOUT FORMULAS DATA REVIEW VIEW novaPDF						
Clipboard Font Alignment Nu						
L12						
	A	C	D	E	F	G
	X [m]	Latitude	Longitude	LAYER-1: Depth [m]	LAYER-2: Depth [m]	LAYER-3: Depth [m]
1						
2						
3	166240	13° 31' 68.989" N	77° 14' 57.611" E	0.082	0.153	0.418
4	166340	13° 31' 74.098" N	77° 14' 59.396" E	0.082	0.156	0.441
5	166440	13° 31' 79.420" N	77° 14' 60.357" E	0.055	0.134	0.461
6	166540	13° 31' 84.736" N	77° 14' 61.318" E	0.059	0.137	0.473
7	166640	13° 31' 90.103" N	77° 14' 62.234" E	0.061	0.152	0.457
8	166740	13° 31' 95.407" N	77° 14' 63.012" E	0.082	0.183	0.462
9	166840	13° 32' 0.769" N	77° 14' 63.653" E	0.059	0.145	0.454
10	166940	13° 32' 6.130" N	77° 14' 64.202" E	0.069	0.191	0.473
11	167040	13° 32' 11.503" N	77° 14' 64.797" E	0.077	0.185	0.465
12	167140	13° 32' 16.928" N	77° 14' 65.255" E	0.059	0.152	0.473
13	167240	13° 32' 22.249" N	77° 14' 65.850" E	0.086	0.186	0.487
14	167340	13° 32' 27.640" N	77° 14' 66.262" E	0.057	0.147	0.398
15	167440	13° 32' 33" N	77° 14' 65.667" E	0.078	0.156	0.453
16	167540	13° 32' 38.357" N	77° 14' 65.026" E	0.059	0.136	0.484
17	167640	13° 32' 43.730" N	77° 14' 64.385" E	0.082	0.168	0.477
18	167740	13° 32' 49.040" N	77° 14' 63.607" E	0.078	0.148	0.480
19	167840	13° 32' 54.379" N	77° 14' 62.600" E	0.070	0.136	0.448
20	167916	13° 32' 58.356" N	77° 14' 61.639" E			
21						

Figure 4-17: Exported Layers Thickness

FILE HOME INSERT PAGE LAYOUT FORMULAS DATA REVIEW VIEW DEVELOP								
Clipboard			Font			Alignment		Number
120								
	A	C	D	E	F	G	H	I
	X [m]	Latitude	Longitude	LAYER-1: Depth [m]	LAYER-2: Depth [m]	LAYER-3: Depth [m]	Thickness (mm)	
1							Bituminous	Granular
2								
3	166240	13° 31' 68.989" N	77° 14' 57.611" E	0.082	0.153	0.418	71	265
4	166340	13° 31' 74.098" N	77° 14' 59.396" E	0.082	0.156	0.441	74	285
5	166440	13° 31' 79.420" N	77° 14' 60.357" E	0.055	0.134	0.461	79	327
6	166540	13° 31' 84.736" N	77° 14' 61.318" E	0.059	0.137	0.473	78	336
7	166640	13° 31' 90.103" N	77° 14' 62.234" E	0.061	0.152	0.457	91	305
8	166740	13° 31' 95.407" N	77° 14' 63.012" E	0.082	0.183	0.462	101	279
9	166840	13° 32' 0.769" N	77° 14' 63.653" E	0.059	0.145	0.454	86	309
10	166940	13° 32' 6.130" N	77° 14' 64.202" E	0.069	0.191	0.473	122	282
11	167040	13° 32' 11.503" N	77° 14' 64.797" E	0.077	0.185	0.465	108	280
12	167140	13° 32' 16.928" N	77° 14' 65.255" E	0.059	0.152	0.473	93	321
13	167240	13° 32' 22.249" N	77° 14' 65.850" E	0.086	0.186	0.487	100	301
14	167340	13° 32' 27.640" N	77° 14' 66.262" E	0.057	0.147	0.398	90	251
15	167440	13° 32' 33" N	77° 14' 65.667" E	0.078	0.156	0.453	78	297
16	167540	13° 32' 38.357" N	77° 14' 65.026" E	0.059	0.136	0.484	77	348
17	167640	13° 32' 43.730" N	77° 14' 64.385" E	0.082	0.168	0.477	86	309
18	167740	13° 32' 49.040" N	77° 14' 63.607" E	0.078	0.148	0.480	70	332
19	167840	13° 32' 54.379" N	77° 14' 62.600" E	0.070	0.136	0.448	66	312
20	167916	13° 32' 58.356" N	77° 14' 61.639" E					

Figure 4-18: Corrected Layers Thickness



4.5 Application of GPR and FWD Results

- The results are used as input for the Analysis of Various Modules in Karnataka Road Asset Management System (KRAMS)
- The surface and base thickness details from GPR results are used for the Surface and base thickness details of KRAMS format whereas remaining data required is obtained from the FWD generated results and the sample KRAMS Format is shown in Table 4-1.



Table 4-1: KRAMS Format- FWD and GPR Results

Road_ID	Road_Name	From	To	Survey_Date	Link_ID	Direction	Lane	Surf_Thick (mm)	Base_Thick (mm)	Surf_Type	Base_Type	SubBase_Thick(mm)	SubBase_Type	CBR(%)	SNP	Disp0_0	Disp1_200	Disp2_300	Disp3_450	Disp4_600	Disp5_900	Disp6_1200	Disp7_1500	Disp8_1800	Air_Temp	Pav_Temp	Pressure	Data_Source
SH2	Highwa	0	500	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	500	1000	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	1000	1500	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	1500	2000	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	2000	2500	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	2500	3000	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	3000	3500	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	3500	4000	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	4000	4500	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	4500	5000	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	5000	5500	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave
SH2	Highwa	5500	6000	19-Sep-16	SRS3	I	L1	60	250	AC	GB	500	SG	6	3.5635	0.8430	0.7430	0.5167	0.3575	0.2622	0.1292	0.0563	0.0381	0.0254	28.97	24.95	700	Pave

GPR Results

FWD Results

- Deflection results collected should be in millimetres and to be normalised for 700Kpa pressure.
- $SNC = 3.2 \cdot d_0^{(-0.63)}$

Where,

SNC = Structural Number Corrected

d_0 = Deflection of the first geophone value in millimetre.



ANNEXURE

Data Compilation Formats with sample data - ROMDAS



Survey Header

SURVEY_ID	SURVEY_FILE	SURVEY DESCRIPTION	SURVEY DATE	VEHICLE	OPERATOR	CHAIN_ START	CHAIN_ END	DIRECTION	LANE	DEVICES	VERSION	LENGTH



Road Data Processing & Quality Assurance Procedures

Planning and Road Asset Management Centre (PRAMC)

GPS

Road_ID	Link_ID	Chainage	Speed	Latitude	Longitude	altitude	Survey_Date	Direction	CRS(Coord.Ref.System)
SH1	UDP203	0	5	13.14082	74.77219	23.600	20-Apr-16	I	UTM WGS1984 ZONE43N
SH1	UDP203	5	9	13.14080	74.77223	23.600	20-Apr-16	I	UTM WGS1984 ZONE43N
SH1	UDP203	10	12	13.14079	74.77227	23.600	20-Apr-16	I	UTM WGS1984 ZONE43N

IRI

Road_ID	Link_ID	From	To	Direction	Lane	LWP_IRI	RWP_IRI	Lane_IRI	Speed	SurveyDate
SH1	UDP203	0	100	I	1	1.12	2.02	1.57	19	20-Apr-16
SH1	UDP203	100	200	I	1	2.12	1.88	2.00	41	20-Apr-16
SH1	UDP203	200	300	I	1	2.72	1.56	2.14	42	20-Apr-16

Road Type

Road_ID	Link_ID	From	To	Direction	Road_Type	SurveyDate
SH1	UDP203	0	10245	I	Double Lane	20-Apr-16
SH1	KRK40	10245	15258	I	Double Lane	20-Apr-16
SH1	KRK40	15258	15556	I	Multi Lane	20-Apr-16

Pavement Width

Road_ID	Link_ID	From	To	Direction	Pave_Width	Road_Width	No of Lanes	SurveyDate
SH1	UDP203	0	10245	I	7	11	2	20-Apr-16
SH1	KRK40	10245	15258	I	7	11	2	20-Apr-16
SH1	KRK40	15258	15556	I	10	14	2.2	20-Apr-16

Surface Type

Road_ID	Link_ID	From	To	Surf_Type	SurveyDate
SH1	UDP203	0	10245	Bituminous	20-Apr-16
SH1	KRK40	10245	15262	Bituminous	20-Apr-16
SH1	KRK40	15262	15564	Bituminous	20-Apr-16



Road Data Processing & Quality Assurance Procedures

Planning and Road Asset Management Centre (PRAMC)

Shoulder Type

Road_ID	Link_ID	From	To	Direction	Shd_Type	SurveyDate
SH1	UDP203	0	769	1	Paved	20-Apr-16
SH1	UDP203	769	3493	1	Paved	20-Apr-16
SH1	UDP203	3493	3743	1	Paved	20-Apr-16

Shoulder Width

Road_ID	Link_ID	From	To	Direction	Shd_Width	SurveyDate
SH1	UDP203	0	87	1	1.5	20-Apr-16
SH1	UDP203	87	752	1	2	20-Apr-16
SH1	UDP203	752	3419	1	1.5	20-Apr-16

Shoulder Elevation

Road_ID	Link_ID	From	To	Direction	Shd_elevation	SurveyDate
SH1	UDP203	0	1672	1	Levelled Shoulder	20-Apr-16
SH1	UDP203	1672	10245	1	Shoulder Drop	20-Apr-16
SH1	KRK40	10245	28185	1	Shoulder Drop	20-Apr-16

Drain Type

Road_ID	Link_ID	From	To	Direction	Drain_Type	SurveyDate
SH1	UDP203	0	506	1	Lined drain	20-Apr-16
SH1	UDP203	506	3276	1	Unlined drain	20-Apr-16
SH1	UDP203	3276	3620	1	Lined drain	20-Apr-16

Rutting

Road_ID	Link_ID	From	To	Direction	Lane	LWP_Rut	RWP_Rut	Lane_Rut	Speed	SurveyDate
SH1	UDP203	0	100	1	1	17.3	16.9	16.9	19	20-Apr-16
SH1	UDP203	100	200	1	1	3.7	3.1	3.1	41	20-Apr-16
SH1	UDP203	200	300	1	1	3.4	3.1	3.1	42	20-Apr-16

Texture

Road_ID	Link_ID	From	To	LWP_MPD	RWP_MPD	Lane_MPD	Survey Date
SH1	UDP203	0	100	1.12	0.74	0.93	20-Apr-16
SH1	UDP203	100	200	0.85	0.57	0.71	20-Apr-16
SH1	UDP203	200	300	0.85	0.64	0.75	20-Apr-16



Road Data Processing & Quality Assurance Procedures

Planning and Road Asset Management Centre (PRAMC)

ESIS: Road Side Amenities

Road Id	Link_ID	Chainage	Land Use Pattern	Survey Date	Road Name
SH1	UDP203	3268	5	20-Apr-16	Highway connecting Padubedri with Chickalaguda Karkala:Agumbe:Thirthahalli:Anandapura:Hangal:Thadasa:Kalghatagi:Dharwad:Uppinabetegeri:Belawadi:Baiahongala:Vannuru:Hoskoti:Ankalagi:Basapura:Hidka I Dam and Hosur
SH1	UDP203	4388	5	20-Apr-16	Highway connecting Padubedri with Chickalaguda Karkala:Agumbe:Thirthahalli:Anandapura:Hangal:Thadasa:Kalghatagi:Dharwad:Uppinabetegeri:Belawadi:Baiahongala:Vannuru:Hoskoti:Ankalagi:Basapura:Hidka I Dam and Hosur
SH1	UDP203	5441	5	20-Apr-16	Highway connecting Padubedri with Chickalaguda Karkala:Agumbe:Thirthahalli:Anandapura:Hangal:Thadasa:Kalghatagi:Dharwad:Uppinabetegeri:Belawadi:Baiahongala:Vannuru:Hoskoti:Ankalagi:Basapura:Hidka I Dam and Hosur

RMMS: Road Furniture

Road_ID	Link_ID	From	To	Direction	RdFur_Type	RdFur_Cond	Repainting	SurveyDate
SH1	UDP203	64	64	I	2	Poor	Y	20-Apr-16
SH1	UDP203	265	265	I	2	Poor	N	20-Apr-16
SH1	UDP203	638	638	I	2	Poor	Y	20-Apr-16

RMMS: Tree

Road ID	Link_ID	From	To	Direction	Tree_Type	Tree_ID	Obstruction_Cond (Major/Minor)	Survey Date
SH1	TRT3	80099	80099	I	3		Poor	20-Apr-16
SH1	TRT3	114804	114804	I	3		Poor	02-May-16
SH1	TRT3	114843	114843	I	3		Poor	02-May-16

**RMMS: Road Marking**

Road ID	Link_ID	From	To	Direction	Edge_Marking_Reqd (Y/N)	CL_Marking_Reqd (Y/N)	Survey Date	Road Marking Type
SH1	UDP203	0	10245	I		Poor	20-Apr-16	1
SH1	UDP203	0	10245	I	Poor		20-Apr-16	3
SH1	KRK40	10245	77021	I		Poor	20-Apr-16	1

RMMS: Median

Road ID	Link_ID	From	To	Direction	Med_Type	Med_Width	Med_Cond	Survey Date
SH1	KRK40	15278	15537	I	3	1	poor	20-Apr-16
SH1	KRK40	45164	45409	I	3	1	poor	20-Apr-16
SH1	KRK40	45477	45526	I	3	1	poor	20-Apr-16



Visual Condition

Road_ID	Link_ID	From	To	Direction	Lane	Bleeding	Cracking	Depression	Edge_Damage	Patching	Potholes	Ravelling	Shoulder_Condition	Shoving	Drainage_Condition	Survey Date
SH1	UDP203	0	500	I	1	Very Good	Very Good	Very Good	Very Good	Very Good	Very Good	Very Good	Good	Very Good	Good	20-Apr-16
SH1	UDP203	500	1000	I	1	Very Good	Very Good	Very Good	Very Good	Very Good	Very Good	Very Good	Good	Very Good	Good	20-Apr-16
SH1	UDP203	1000	1500	I	1	Very Good	Very Good	Very Good	Very Good	Very Good	Very Good	Very Good	Good	Very Good	Bad	20-Apr-16

LRP

Road_ID	Link_ID	LRP_ID	LRP_Name	Type	Description	Offset	Prev_LRP	LRP_Offset	Diagram	Photo	Latitude	Longitude	Altitude	StartDate	EndDate
SH1	UDP203	LRP_SH1_0	NH 66 Jn padubidari	INT	NH 66 Jn padubidari	0	LRP_SH1_0	0	NULL	NULL	13.140824	74.772194	23.600	20-Apr-16	20-Apr-16
SH1	KRK40	LRP_SH1_33789	Kadu road Jn	INT	Kadu road Jn	33789	LRP_SH1_0	33789	NULL	NULL	13.232850	74.985044	81.400	20-Apr-16	20-Apr-16

Vertical Geometry

Road_ID	Link_Id	From	To	Direction	Geometry Type	Speed	Gradient(%)	Curvature(deg/km)	Camber(%)	RF(m/km)	Num_RF	Altitude	Super Elevation(%)	Cross_slope(%)	Survey Date
SH1	UDP203	0	100	I	Plain	10	-1.10000		2.5	-1.10	1	23.600	2.5	5	20-Apr-16
SH1	UDP203	100	200	I	Plain	32	2.70000		2.5	2.70	1	22.500	2.5	5	20-Apr-16